This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document –

The documents contained herein were originally issued and sealed by the individuals whose names and license numbers appear on each page, on the dates appearing with their signature on that page. This file or an individual page shall not be considered a certified document.



PROIECT LENG			Prepared in the Office	of Mott MacDonald for
			DIVISI NORTH CAROLINA DEPARTM	UN 5 MENT OF TRANSPORTATION
NGTH ROADWAY TIP PROJECT	=	0.103 MILES	2018 STANDARD SPECIFICATIONS RIGHT OF WAY DATE:	TIM JORDAN, PE
NGTH STRUCTURE TIP PROJECT	=	0.011 MILES	NOVEMBER 20, 2017	PROJECT ENGINEER
TAL LENGTH TIP PROJECT	=	0.114 MILES	LETTING DATE: AUGUST 11, 2021	JAMES R. RICE, PE HYDRAULICS ENGINEER
			NCDOT CONTACT:	LISA GILCHRIST, EI

STATE STAT	E PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS		
N.C. 17	BP.5.R.70	1			
STATE PROJECT NO.	F. A. PROJ. NO.	DESCRIPTION			
17BP.5.R.70		PE			
17BP.5.R.70		ROW /UTIL CONST			
17BP.5.R.70					

GENERAL NOTES: GRADING AND S THE SURF ADJU ENG CLEARING: CL	2018 SPECIFICAT JRFACING: GRADE LINES SHOWN DENOTE ACING AT GRADE POINTS SHO ISTED AT THEIR BEGINNING AI NEER IN ORDER TO SECURE A RING ON THIS PROJECT SHAL IOD III. CURVES ON THIS PROJECT SHAL ION IS TO BE REVOLVE ION IS IN	TIONS E THE FINIS OWN ON ND ENDING A PROPER 1 L BE PERFO L BE PERFO	FFECTIVE: 01–16–18 HED ELEVATION OF THE PR THE TYPICAL SECTIONS. GRA 3 AND AT STRUCTURES AS TE–IN. RMED TO THE LIMITS ESTAR RELEVATED IN ACCORDANC ION AND RUNOFF SHOWN THE GRADE POINTS SHOWN	COPO DE L DIRE BLISH
GRADING AND S THE SURE ADJU ENG CLEARING: CLEARIN	JRFACING: GRADE LINES SHOWN DENOTE ACING AT GRADE POINTS SHO ISTED AT THEIR BEGINNING AI NEER IN ORDER TO SECURE A RING ON THIS PROJECT SHAL IOD III. CURVES ON THIS PROJECT SHAL 225.04 USING THE RATE OF S RELEVATION IS TO BE REVOLV IONS. RUCTION: ALT, EARTH, AND CONCRETE SI RELEVATED CURVES SHALL BE I	E THE FINIS OWN ON ND ENDING A PROPER 1 .L BE PERFO ALL BE SUPE UPERELEVAT /ED ABOUT	HED ELEVATION OF THE PR THE TYPICAL SECTIONS. GRA 3 AND AT STRUCTURES AS TE-IN. RMED TO THE LIMITS ESTAG RELEVATED IN ACCORDANC TON AND RUNOFF SHOWN THE GRADE POINTS SHOWN	CPO DE L DIRE BLISH
CLEARING: CLEARI	GRADE LINES SHOWN DENOTE ACING AT GRADE POINTS SHO ISTED AT THEIR BEGINNING AN NEER IN ORDER TO SECURE A RING ON THIS PROJECT SHAL IOD III. CURVES ON THIS PROJECT SHA 225.04 USING THE RATE OF S RELEVATION IS TO BE REVOLV IONS. RUCTION: ALT, EARTH, AND CONCRETE SI RELEVATED CURVES SHALL BE I	E THE FINIS OWN ON ND ENDING A PROPER 1 L BE PERFO L BE SUPE UPERELEVAT /ED ABOUT	HED ELEVATION OF THE PR THE TYPICAL SECTIONS. GRA G AND AT STRUCTURES AS TE-IN. RMED TO THE LIMITS ESTAG RELEVATED IN ACCORDANC TON AND RUNOFF SHOWN THE GRADE POINTS SHOWN	ROPC DE L DIRE BLISH
CLEARING: CLEARING:	RING ON THIS PROJECT SHAL IOD III. CURVES ON THIS PROJECT SHA 225.04 USING THE RATE OF S RELEVATION IS TO BE REVOLV IONS. RUCTION: ALT, EARTH, AND CONCRETE SI RELEVATED CURVES SHALL BE I	L BE PERFO	RMED TO THE LIMITS ESTAG RELEVATED IN ACCORDANC ION AND RUNOFF SHOWN THE GRADE POINTS SHOWN	BLISH CE W 1 ON N O
CLEA METH SUPERELEVATION: ALL NO, SUPE SUPE SECT SHOULDER CONST ASPH SUPE GUARDRAIL: THE CON WITH SUBSURFACE PLAN NO, MAK END BENTS:	RING ON THIS PROJECT SHAL IOD III. CURVES ON THIS PROJECT SHA 225.04 USING THE RATE OF S RELEVATION IS TO BE REVOLV IONS. RUCTION: ALT, EARTH, AND CONCRETE SI RELEVATED CURVES SHALL BE I	L BE PERFC ALL BE SUPE SUPERELEVAT /ED ABOUT	RMED TO THE LIMITS ESTAG RELEVATED IN ACCORDANC ION AND RUNOFF SHOWN THE GRADE POINTS SHOWN	BLISH CE W I ON N O
SUPERELEVATION: ALL NO. SUPE SECT SHOULDER CONST ASPH SUPE GUARDRAIL: THE CON WITH SUBSURFACE PLAN NO MAK END BENTS:	CURVES ON THIS PROJECT SHA 225.04 USING THE RATE OF S RELEVATION IS TO BE REVOLV IONS. RUCTION: ALT, EARTH, AND CONCRETE SI RELEVATED CURVES SHALL BE I	ALL BE SUPE SUPERELEVAT /ED ABOUT HOULDER C	RELEVATED IN ACCORDANC ION AND RUNOFF SHOWN THE GRADE POINTS SHOWN	W 3 10 1 0 1
ALL NO. SUPE SECT SHOULDER CONST ASPH SUPE GUARDRAIL: THE CON WITH SUBSURFACE PLAN NO MAK END BENTS: THE SECT	CURVES ON THIS PROJECT SHA 225.04 USING THE RATE OF S RELEVATION IS TO BE REVOLV IONS. RUCTION: ALT, EARTH, AND CONCRETE SI RELEVATED CURVES SHALL BE I	ALL BE SUPE SUPERELEVAT /ED ABOUT HOULDER C	RELEVATED IN ACCORDANC ION AND RUNOFF SHOWN THE GRADE POINTS SHOWN	W 30 10 10 10
SHOULDER CONST ASPH SUPE GUARDRAIL: THE CON WITH SUBSURFACE PLAN NO MAK END BENTS: THE SECT	RUCTION: ALT, EARTH, AND CONCRETE SI RELEVATED CURVES SHALL BE I	HOULDER C		
GUARDRAIL: GUARDRAIL: SUBSURFACE PLAN NO MAK END BENTS: THE SECT	ALT, EARTH, AND CONCRETE SI RELEVATED CURVES SHALL BE I	HOULDER C		
GUARDRAIL: THE CON WITH SUBSURFACE PLAN NO MAK END BENTS: THE SECT		IN ACCORE	CONSTRUCTION ON THE HI DANCE WITH STD. NO. 560.0	GH)1.
SUBSURFACE PLAN NO MAK END BENTS: THE SECT				
SUBSURFACE PLAN NO MAK END BENTS: THE SECT APPR	GUARDRAIL LOCATIONS SHOW STRUCTION AS DIRECTED BY THE ENGINEER PRIOR TO OR	n on the the engini rdering g	E PLANS MAY BE ADJUSTED EER. THE CONTRACTOR SHO UARDRAIL MATERIAL.	DUR VULD
NO MAK END BENTS: THE SECT APPR	S:			
END BENTS: THE SECT APPR	SUBSURFACE PLANS ARE AVAIL E HIS OWN INVESTIGATION A	ABLE ON T	THIS PROJECT. THE CONTRAC SUBSURFACE CONDITIONS.	TOR
THE SECT APPR				
	SURVEYOR SHALL CHECK THE S IONS PRIOR TO SETTING OF OACHING A BRIDGE.	STRUCTURE THE SLOPE	END BENT PLANS, DETAILS, A STAKES FOR THE EMBANKME	AND ENT
UTILITIES:				
UTIL	TY OWNER ON THIS PROJECT	IS WAKE E	MC.	
RIGHT-OF-WAY M	ARKERS:			
ALL	RIGHT-OF-WAY MARKERS ON 1	THIS PROJE	CT SHALL BE PLACED BY CO)NTR/

2/2021 7:54:12 AM 308350_DDC.12\DI_17BP.5.R.70\Roadway\Proj\340028_rdy_psh1A.dgn 366165

 \square

	LIST OF ROADWAY STANDARD
	2018 ROADWAY ENGLISH STANDARD DRAWINGS
	The following Roadway Standards as appear in "Roadway Standard Drawings" Highv N. C. Department of Transportation – Raleigh, N. C., Dated January, 2018 are applice and by reference hereby are considered a part of these plans:
ROPOSED ADE LINES MAY BE	STD.NO. TITLE
DIRECTED BY THE	DIVISION 2 – EARTHWORK
	200.03 Method of Clearing – Method III
	225.02 Guide for Grading Subgrade – Secondary and Local
	225.04 Method of Obtaining Superelevation – Two Lane Pavement
BLISHED BY	DIVISION 3 - PIPE CULVERTS
	300.01 Method of Pipe Installation
	DIVISION 4 – MAJOR STRUCTURES
	422.02 Bridge approach Fills, Type II Modified Approach Fills
CE WITH STD.	
N ON THE PLANS.	DIVISION 5 – SUBGRADE, BASES AND SHOULDERS
N ON THE TYPICAL	560.01 Method of Shoulder Construction – High Side of Superelevated Curve –
	DIVISION 8 – INCIDENTALS
	806.01 Concrete Right-of-way Marker
	806.02 Granite Right-of-way Marker
IGH SIDE OF	840.00 Concrete Base Pad for Drainage Structures
01.	840.25 Anchorage for Frames – Brick or Concrete or Precast
	840.29 Frames and Narrow Slot Flat Grates
	840.35 Traffic Bearing Grated Drop Inlet – for Cast Iron Double Frame and Gr
	840.46 Traffic Bearing Precast Drainage Structure
	840.66 Drainage Structure Steps
DULD CONSULI	846.01 Concrete Curb, Gutter and Curb & Gutter
	846.04 Drop Inlet Installation in Shoulder Berm Gutter
	862.01 Guardrail Placement
	862.02 Guararali Installation 862.03 Structure Anchor Unite
	876.01 Rin Ran in Channels
	876.02 Guide for Rin Ran at Pine Outlets
AND CROSS-	
ENT OR EXCAVATION	INDEX OF SHEETS

SHEET NUMBER	DESCRIPTION
1	TITLE SHEET
1A	INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STA
1B	CONVENTIONAL SYMBOLS
2A–1	PAVEMENT SCHEDULE AND TYPICAL SECTIONS
2C–1	GUARDRAIL INSTALLATION
2C-2	STRUCTURE ANCHOR UNITS
3B–1	GUARDRAIL SUMMARY, SHOULDER BERM GUTTER SUM BRIDGE WAITING PERIODS, AND EARTHWORK SUMMA
3D-1	DRAINAGE SUMMARY
4	PLAN SHEET
5	PROFILE SHEET
TMP–1 THRU TMP–5	TRAFFIC MANAGEMENT PLANS
EC–1 THRU EC–5	EROSION CONTROL PLANS
RF–1	REFORESTATION DETAIL SHEET
UO–1 THRU UO–2	UTILITIES BY OTHERS PLANS
X–1A	CROSS-SECTION SUMMARY
X–1 THRU X–3	CROSS-SECTIONS
S–1 THRU S–15	STRUCTURE PLANS
SN	STANDARD STRUCTURE NOTES

.70 - FRANKLIN 28 AY DESIGN GINEER CAROL SSION EAL 1102 MENT NOT CONSI S ALL SIGNATURE he M M M M M T ⁷⁶² Fug MACDONALD Fug M M M M M M M M M M M M M	IA IDERED FINAL ES COMPLETED Purfoy Road, Suite 115 puay-Varina, NC 27526 w.mottmac.com/americas
GINEER CAROL SSION EAL 1102 ONALD I& E, LLC E NO. F-0669 MENT NOT CONSIST S ALL SIGNATURE he MOTT M 762 Fuq MACDONALD WWW	DERED FINAL S COMPLETED
CAROL SS JON EAL 1102 ONALD 1& E, LLC E NO. F-0669 JMENT NOT CONSI S ALL SIGNATURE he M MOTT MA MACDONALD 762 Fuq MACDONALD 762	DERED FINAL S COMPLETED
SEAL 1102 WETH ONALD 1& E, LLC E NO. F-0669 MENT NOT CONSISS ALL SIGNATURE he M M M M M M M M M M M M M	DERED FINAL S COMPLETED
ONALD 1& E, LLC E NO. F-0669 JMENT NOT CONSISS ALL SIGNATURE he MOTT MACDONALD 762 MACDONALD 762	DERED FINAL S COMPLETED
ONALD 1& E, LLC E NO. F-0669 MENT NOT CONSI S ALL SIGNATURE he MOTT MACDONALD 762 Fuq MACDONALD 762 Fuq	DERED FINAL S COMPLETED
ONALD I& E, LLC E NO. F-0669 JMENT NOT CONSISS ALL SIGNATURE he M MOTT MACDONALD 762 Fuq MACDONALD WWW	Profession Contraction Contrac
The M MacDonald Function MacDonald Function MacDona	Professional Suite 115 Sugar-Varina, NC 27526 w.mottmac.com/americas
he M MACDONALD Fug	Purfoy Road, Suite 115 Jugy-Varina, NC 27526 w.mottmac.com/americas
he M MOTT M Fuq MACDONALD www	1 Purfoy Road, Suite 115 uay–Varina, NC 27526 w.mottmac.com/americas
MACDONALD Y	

BOUNDARIES AND PROPERTY:

State Line

Township Line	
City Line	
Reservation Line	· ·
Property Line	
Existing Iron Pin	EIP
Computed Property Corner	
Property Monument	ECM
Parcel/Sequence Number	— (123)
Existing Fence Line	
Proposed Woven Wire Fence	
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary	— — — — WLB— — — —
Proposed Wetland Boundary	
Existing Endangered Animal Boundary	— — EAB — — — — — — — — — — — — — — — — — — —
Existing Endangered Plant Boundary	— — EPB — — — — — — — — — — — — — — — — — — —
Existing Historic Property Boundary	— нрв ———
Known Contamination Area: Soil	😿 - s 😿 -
Potential Contamination Area: Soil	<u>)</u> - s - <u>)</u>
Known Contamination Area: Water	😿 W 😿 -
Potential Contamination Area: Water	j?; -w-j?; -
Contaminated Site: Known or Potential ——	-).
BUILDINGS AND OTHER CULT	URE:
Gas Pump Vent or U/G Tank Cap	- 0
Sign	– <u>O</u>
Sign Well	- O S W
Sign Well Small Mine	- [©] s - [©] w - [×] ×
Sign Well Small Mine Foundation	- [©] s - [©] w - [×]
Sign Well Small Mine Foundation Area Outline	- [©] s - [©] w - [×] ×
Sign Well Small Mine Foundation Area Outline Cemetery	- [©] s - [©] s - [×] - [×]
Sign Well Small Mine Foundation Area Outline Cemetery Building	
Sign Well Small Mine Foundation Area Outline Cemetery Building School	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church	
Sign Well Small Mine Foundation Foundation Area Outline Cemetery Building School Church Dam	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY:	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream	
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1	$ = \qquad $
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2	$ \begin{array}{c} & & & & \\ & & & & \\ & & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ $
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow	$ \begin{array}{c} & & & & \\ & & & & \\ & & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ $
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream	$ = \qquad \bigcirc \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\$
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring	$ \begin{array}{c} \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring Wetland	$ \begin{array}{c} $
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring Wetland Proposed Lateral, Tail, Head Ditch	$ = \qquad \bigcirc S \\ S \\ = \qquad \bigcirc W \\ = \qquad & \swarrow \\ W \\ = \qquad & \bigcirc \\ W \\ = \qquad & & \\ W \\ = \qquad & \\$
Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam <i>HYDROLOGY:</i> Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring Wetland Proposed Lateral, Tail, Head Ditch False Sump	$ = \qquad $

Νü

RAILROA

Standard Go RR Signal Mi Switch —— RR Abandor **RR** Dismant

RIGHT

Secondary Primary Ho Primary Ho Exist Perma New Perm Vertical Ben Existing Rig Existing Rig New Right New Right New Right Concret New Cont Concret Existing Co New Cont Existing Ea New Tem New Temp New Perm New Perm New Perm New Temp New Aeria

ROADS

Existing E Existing C Proposed Proposed Proposed Existing A Proposed Existing C Proposed Equality S Pavement VEGET Single Tre Single Sh

auge	CSX TRANSPORTATION	Hedge	
Ailepost	MILEPOST 35	Woods Line	
	SWITCH	Orchard	
ned	_++++_	Vineyard	Vineyard
iled		EXISTING STRUCTURES:	
		MAJOR:	
OF WAY & PROJECT CO	NIROL:	Bridge, Tunnel or Box Culvert	CONC
Horiz and Vert Control Point ——	$\bigoplus_{i=1}^{n}$	Bridge Wing Wall, Head Wall and End Wall –	CONC WW
oriz Control Point		MINOR:	
oriz and Vert Control Point	<u> </u>	Piece Cubuert	/ CONC HW \
anent Easment Pin and Cap ———	$\langle \cdot \rangle$	Pipe Cuivert	
nanent Easement Pin and Cap ——	$\mathbf{\bullet}$	Footbridge	
nchmark		Drainage Box: Catch Basin, DI or JB ———	СВ
ght of Way Marker	\bigtriangleup	Paved Ditch Gutter	
ght of Way Line		Storm Sewer Manhole	(\mathbb{S})
t of Way Line	$(\overset{\kappa}{W})$	Storm Sewer	S
t of Way Line with Pin and Cap—		UTILITIES:	
t of Way Line with	\overline{R}	POWER:	
te or Granite R/W Marker		Existing Power Pole	
te C/A Marker		Proposed Power Pole	6
ontrol of Access	(<u>C</u>)	Existing Joint Use Pole	
trol of Access		Proposed Joint Use Pole	-0-
asement Line	Α	Power Manhole	P
porary Construction Easement –	F	Power Line Tower	\boxtimes
porary Drainage Easement	- TDF	Power Transformer	\swarrow
nanent Drainage Easement		U/G Power Cable Hand Hole	
nanent Drainage / Utility Fasement		H-Frame Pole	••
nanent Utility Easement		U/G Power Line LOS B (S.U.E.*)	— — — P— — – –
porary Utility Easement	TUE	U/G Power Line LOS C (S.U.E.*)	—— — P — — —
al Utility Easement	ΔI IE	U/G Power Line LOS D (S.U.E.*)	P
	7.02	TELEPHONE	
AND RELATED FEATURE	E S:		
lge of Pavement		Existing Telephone Pole	
urb		Proposed Telephone Pole	- O -
Slope Stakes Cut	<u>C</u>	Telephone Manhole	(T)
Slope Stakes Fill	<u>F</u>	Telephone Pedestal	Ш т
Curb Ramp	CR	Ielephone Cell Iower	√● y
etal Guardrail		U/G Telephone Cable Hand Hole	НН
		U/G Telephone Cable LOS B (S.U.E.*)	T

Edge of Pavement	
Curb	
Slope Stakes Cut	<u>C</u>
Slope Stakes Fill	F
Curb Ramp	CR
Netal Guardrail	<u> </u>
Guardrail	<u> </u>
Cable Guiderail	
Cable Guiderail	
Symbol	\bullet
t Removal	
TATION:	
ee	- £:
nrub	- £3

A DIVISION OF HIGHWA	VS	PROJECT REFE 17BP.5.R.70 – FRA	RENCE SHEET NKLIN 28 11			
NI CLIEET CV/AAD			1			
IN SIRE I SIMBC	JL3	WATER				
E. = Subsurface Utility Engineering		Water Manholo	— W			
edge	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	Water Meter				
oods Line			_ &			
rchard	සි සි සි සි	Water Wulkant	- Å			
neyard	Vineyard	Water Hyarant				
EXISTING STRUCTURES:		U/G water line LOS B (S.U.E [*])				
AJOR:		U/G water Line LOS C (S.U.E*)				
ridge, Tunnel or Box Culvert	CONC	U/G water Line LOS D (S.U.E*)	A/G Water			
ridge Wing Wall, Head Wall and End Wall -	-) CONC WW (Above Ground Water Line				
NOR:		TV:				
lead and End Wall	CONC HW	TV Pedestal				
ipe Culvert		TV Tower	- 🛞			
ootbridge	≻≺	U/G TV Cable Hand Hole	— H _H			
rainage Box: Catch Basin. DI or JB	СВ	U/G TV Cable LOS B (S.U.E.*)	TV			
aved Ditch Gutter		U/G TV Cable LOS C (S.U.E.*)				
torm Sewer Manhole	(S)	U/G TV Cable LOS D (S.U.E.*)	TVTV			
torm Sewer	s	U/G Fiber Optic Cable LOS B (S.U.E.*) —				
		U/G Fiber Optic Cable LOS C (S.U.E.*)	TV F0			
TILITIES:		U/G Fiber Optic Cable LOS D (S.U.E.*)	TV F0			
OWER:		GAS:				
xisting Power Pole	•	Gas Valve	- 🔷			
roposed Power Pole	6	Gas Meter	$ \Diamond$			
xisting Joint Use Pole		U/G Gas Line LOS B (S.U.F.*)	G			
roposed Joint Use Pole	-0-	U/G Gas Line LOS C (SUE*)	G			
ower Manhole	P	U/G Gas Line LOS D (SUE*)	G			
ower Line Tower	\boxtimes	Above Ground Gas Line	A/G Gas			
ower Transformer	\square					
VG Power Cable Hand Hole		SANITARY SEWER:				
I-Frame Pole	••	Sanitary Sewer Manhole	-			
VG Power Line LOS B (S.U.E.*)	— — — P — — — —	Sanitary Sewer Cleanout	- +			
VG Power Line LOS C (S.U.E.*)	——————————————————————————————————————	U/G Sanitary Sewer Line	SS			
VG Power Line LOS D (S.U.E.*)	P	Above Ground Sanitary Sewer	A/G Sanitary Sewer			
		SS Forced Main Line LOS B (S.U.E.*) ——	— — — — FSS — — —			
		SS Forced Main Line LOS C (S.U.E.*)	— — FSS — — —			
xisting Telephone Pole	-•-	SS Forced Main Line LOS D (S.U.E.*)	FSS			
roposed Telephone Pole	-0-					
elephone Manhole	\bigcirc	MISCELLANEOUS:				
elephone Pedestal	\Box	Utility Pole	-			
elephone Cell Tower	, Ť,	Utility Pole with Base				
VG Telephone Cable Hand Hole ———	HH	Utility Located Object	- •			
G Telephone Cable LOS B (S.U.E.*)	T	Utility Traffic Signal Box	— S			
/G Telephone Cable LOS C (S.U.E.*) ——	T	Utility Unknown U/G Line LOS B (S.U.E.*)				
G Telephone Cable LOS D (S.U.E.*)	T	U/G Tank; Water, Gas, Oil	—			
G Telephone Conduit LOS B (S.U.E.*)	— — — — TC— — — –	Underground Storage Tank, Approx. Loc. —	UST)			
/G Telephone Conduit LOS C (S.U.E.*)	TC	A/G Tank; Water, Gas, Oil				
/G Telephone Conduit LOS D (S.U.E.*)	тс	Geoenvironmental Boring	- 📀			
VG Fiber Optics Cable LOS B (S.U.E.*)	— — — T FO— —	U/G Test Hole LOS A (S.U.E.*)	-			
J/G Fiber Optics Cable LOS C (S.U.F *)	T FO	Abandoned According to Utility Records —	– AATUR			
J/G Fiber Ontice Cable LOS D (SILE *)	T FO	End of Information	– E.O.I.			

DocuSign Envelope ID: 6106DE7C-83C5-46AE-9B04-B23800A4C5E3



USE TYPICAL SECTION NO. 2: -L- STA 13+67.88 (BEGIN BRIDGE) TO 14+25.13 (END BRIDGE)

NOTE: SEE STRUCTURE PLANS FOR PAVEMENT DEPTHS ON STRUCTURE

≥α ⊥∧ 400 400





С	P A
C1	P A
C2	P A B
D	P I
Е	P A
R	S
Т	E
U	E
NOTE: P	AVEN

PAVEMENT SCHEDULE

PROP. APPROX. $1\frac{1}{2}$ " ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS. PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT GREATER THAN $1\frac{1}{2}$ " IN DEPTH. PROP. APPROX. 4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE [19.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD. PROP. APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD. SHOULDER BERM GUTTER. EARTH MATERIAL. EXISTING PAVEMENT.

MENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.



•





																						SHEET NO
TOTAL SHOULD	DER WIDTH = DISTAN	ARE TO FACE OF GUA	AVEL LANE TO SHOUL	DER BREAK POINT	Г.																	
FLARE LENGTH	= DISTANCE FROM	LAST SECTION OF PAR	ALLEL GUARDRAIL TO	ND OF GUARDR	AIL.																	
W = TOTAL G = GATING	WIDTH OF FLARE FRO	M BEGINNING OF TAP R TYPE 350	ER TO END OF GUAR	DRAIL.					ΠΓ	\mathbf{D}		C	ΤΙλ	Γλ	Δ₽	\mathbf{V}						
NG = NON-	GATING IMPACT ATTER	NUATOR TYPE 350						UUA					$\mathbf{O}\mathbf{I}\mathbf{V}$									
															Ι					ІМРАСТ]
SURVEY LINE					LENGTH		WARRANT POINT		"N" DIST.	TOTAL	FLARE	LENGTH		W		ANCHORS ATTENUATO						
	BEG. STA.	END STA.	LOCATION		SHOP	DOUBLE		TRAILING	FROM	SHOULDER WIDTH		TRAILING		TRAILING		CPELL					REMARKS	
				STRAIGHT	CURVED	FACED	END	END	1.0.1.		END	END	END	END	AT_1	TL-3	TYPE III			NO. G NO	-	
L	12+86.63	13+67.88	RT	81.25′				13+67.88	4'	7′	50′		1′			1	1					
-L-	12+86.63	13+67.88	LT	81.25′			13+67.88		4'	7'		50′		1′		1	1					
-L-	14+25.13	15+31.38	RT	106.25'			14+25.13		4'	7'		50′		1′		1	1					
-L-	14+25.13	15+06.38	LT	81.25′				14+25.13	4'	7'	50′		1′			1	1					
		SUBT	OTAL	350.00′																		
		LESS ANCHO	r deductions																			
		GREU, TL–3	4 x 50.00' =	-200.00'																		
		TYPE III	4 x 18.75′ =	-75.00'																		
				75.00/												4						
				/5.00												4	4				ADDITIONAL GUARDRAIL POST = 3 EA	

SUMMARY OF EXISTING ASPHALT PAVEMENT REMOVAL

SURVEY LINE	STATION	STATION	LOCATION LT/RT/CL	SY
-L-	12 + 00.00	13+84.82	CL	407.94
-L-	14+25.84	16+50.00	CL	513.53
			TOTAL:	921.46
			SAY:	970

SHOULDER BERM **GUTTER SUMMARY**

SURVEY LINE	BEG. STA.	END STA.	LENGT
-L- RT	14+36.00	14+80.00	44.00
		TOTAL	44.00
		SAY	50.00

LOCATION	UNCLASSIFIED EXCAVATION	UNDERCUT	EMBT + %	BORROW	WASTE
-L- 12+00.00 TO 13+67.88 (BEGIN BRIDGE)	76		115	39	
-L- 14+25.13 (END BRIDGE) TO 16+50.00	81		378	297	
SUBTOTAL	157		493	336	
WASTE IN LIEU OF BORROW					
EST. SHOULDER MATERIAL				127	
PROJECT TOTAL	157		493	463	
5% TO REPLACE BORROW				24	
GRAND TOTAL	157		493	487	
SAY	170			520	

EST. 75 CY SELECT GRANULAR MATERIAL (CONTINGENCY) EST. 50 CY UNDERCUT (CONTINGENCY) EST. 40 CY DDE (SEE PLAN SHEET 4)

NOTE: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, Clearing and Grubbing and Removal of Existing

SUMMARY OF BRIDGE WAITING PERIODS

BRIDGE DESCRIPTIONS

BRIDGE NO. 28 ON SR 1210 (MONTGOMERY RD.) OVER BILLY'S CREEK

END BENT/ BENT NO.	MONTHS
END BENT 1	1

																												PROJECT REFERENCE 7BP.5.R.70 – FRANKLIN 28	SHEET NO. 3D–1
									R_F	2F(7 <i>1(</i>) NI	A 1	.	-	Rŀ	i C	Π	NAI										
		T	JST	' OF	F J	ΡΓΡ	ES			VA VA	I.I.	S.	E1		, (F	'NR			PES	42	? "	6	1	IN	DF	CR))		
							20,					.				NDWALLS	AY COL: 'B')												
			DR/	AINAGE PIPE		C.	S. PIPE		R.C. PIPE (CLASS III)		R (C	.C. PIPE LASS IV)		SIGN PIPE	STI	D. 838.01, D. 838.11	k jikainagi Tructures L L.F. For F Y Shall Be (1.3 X Col			CONCRETE	SECTION	140.22 0.24	STD. 840.24 STD. 840.29			۲۲.0		ABBREVIATIO C.B. CATCH BASIN	15
T,RT, OR CL	NO E	AL	(RCP, CSP, 1	CAAP, HDPE, or PVC;	,)									RACTOR DE	STI (OR D. 838.80 UNLESS NOTED HERWISE)	S * TOTA * TOTA QUANTIT * +	40.02	FRAME, GRATES AND HOOD STANDARD 840.03	TRO	D. 840.22	ATES STD. 8 ATE STD. 84	D GRATES	SIZE	STD 840.72	Y. STD. 840		N.D.I.NARROWDROPD.I.DROPINLETG.D.I.GRATEDDROPIIIDROPIII	
SIZE SIZE	DP ELEVATIC		." 15" 18" 24" 30)" 36" 42" 48" <u> </u>	P AP	12" 15" 18"	24" 36" 42"	48" 15" 18"	24" 30" 36"	42" 48" 12	" 15" 18" 2-	4″ 30″ 36″	42" 48"	SS V) ERTS, CONT ERTS, CONT		CU. YDS.	LIN. *FT. A B	OR STD. 8			H GRATE ST	H TWO GR	WITH TWO	vs NO. &	CL. "B" C.Y.	IPE PLUG, C	l.FT.	G.D.I. (N.S.) GRATED DROP II (NARROW SLOT) J.B. JUNCTION BOX M.H. MANHOLE	ILET
THICKNESS OR GAUGE	₽ ≤					064 064 064)64)79 109	60						C. PIPE (CLA)		S.P. CH (0' THR	RU 10.0' ND ABOVE	D. 840.01	TYPE OF GRATE	H BASIN	Inlet Frame with	FRAME WITH	(N.S.) FRAMI (N.S.) FRAMI	. STD. 840.3 STEEL ELBO	. COLLARS	. & BRICK F	EMOVAL LIN	T.B.D.I. TRAFFIC BEARING T.B.J.B. TRAFFIC BEARING	DROP INLET
14+75 RT 401 402 2	223.90 221.10	219.50												*** R. O	CI S "ci Si Ci Si		5.0' TH	C.B. STI	E F G	CATCH	G.D.I.	G.D.I. 1 G.D.I.	G.D.I. G.D.I.	1. 2@15"	CONC	CONC	PIPE RI	REMARKS	
																							· ·						
NOTE: Invert Elevations of	are for Bid F	Purposes only	y and shall no	ot be used for	project con	12' Istruction st	akeout.									1							1	1 2@15"					
See "Standard S	pecifications	For Roads a	nd Structures,	Section 300-	5″.																								

DocuSign Envelope ID: 6106DE7C-83C5-46AE-9B04-B23800A4C5E3

5:10:31 PM -0J\340028-

	PROJECT REFERENCE	SHEET NO.
	ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
	TH CAROL	NOTH CAROL
BRIDE HURANA BRIDE BRIDE HURANA 240 230 230 240 230 100 100	NOK FESSION T	HON FESSION T
CULTURE AND CONTRACTOR OF THE CONTRACT OF		SEAL Docusignee@31986
United and a state of sta	Janas Simother brass	Jazepptopptersan 12202 HES R. R. Ct. 19
DOCUMENT NOT CONSIDERED FINAL Density in the intervention Marked States and a second	MOTT MACDONALD 1& E, LLC LICENSE NO. F-0669	HDR ENGINEERING LICENSE NO. F-0116
Proved in Your Minimum Contraction of the second of th	DOCUMENT NOT CONS UNLESS ALL SIGNATUR	SIDERED FINAL
BRIDSE HYDRAUUC DATA 220 230 230 230 240 230 250 210 210 230 220 210 210 210 210 200 190 190 190 150 190 150 190 150	Prepared in the Office of: M	621 Purfov Road. Suite 115
ERIDGE HYDRAULIC DATA DESIGN FICULATION ERIDGE HYDRAULIC DATA DESIGN MIX ELEVATION ESGN FICULATION ESGN	MOTT FU MACDONALD W	Jquay—Varina, NC 27526 ww.mottmac.com/americas
Verticul Sole 2000000000000000000000000000000000000	HDR Enginee	ering, Inc. of the Carolinas St, Suite 900 Raleigh, N.C. 27601
Image Image <td< td=""><td>VERTICAL SCALE</td><td>HORIZONTAL SCALE</td></td<>	VERTICAL SCALE	HORIZONTAL SCALE
240 230 220 210 200 190 190 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE = 1600 CF5 DESIGN FREQUENCY = 1600 190 180 170 185 185 185 185 150 0/ERTOPHING ELEVATION 2248 FT 150 0/ERTOPHING ELEVATION 2248 FT 150 0/ERTOPHING FEQUENCY = 3-1-2017 W.S.LELVATION 130		, 0 25' 50'
240 230 230 220 210 210 200 190 190 180 190 180 170 150 00 CFS 150 00 CFS 150 150 150 150 150 150 150 150		
240 230 220 210 210 200 190 180 190 180 170 BRIDCE HYDRAULIC DATA DESIGN DISCHARGE = 1600 CFS DESIGN FREQUENCY = 25 YFS 160 DESIGN FREQUENCY = 244 FT BASE TREQUENCY = 00 YFS BASE TREQUENCY = 00 YFS 150 OVERTOPING FREQUENCY = 00+- YRS OVERTOPING FREQUENCY = 00+- YRS ISON		
240 230 220 210 210 200 190 180 190		
240 230 220 210 210 200 190 190 180 170 BRIDGE HYDRAULIC DATA 223 DESIGN DISCHARGE - 1600 DESIGN MISCHARGE - 253 BASE DISCHARGE - 1600 DESIGN MISCHARGE - 223.8 DESIGN MISCHARGE - 244 DESIGN MISCHARGE - 244 DESIGN MISCHARGE - 246.6 DASE INVELEVATION - 224.6 DASE INVELEVATION - 224.6 DASE INVELEVATION - 224.6 DASE OF SURVEY - 30.0 DATE OF SURVEY - 3.1-2017 W.S.ELEVATION - 224.4 FT DATE OF SURVEY - 3.1-2017 W.S.ELEVATION - 224.5 FT 140 - 130		
240 230 220 210 210 200 190 190 190 180 180 170 180 180 180 170 180 180 170 180 180 170 180 180 170 180 180 170 180 180 170 180 180 170 180 180 180 180 180 180 180 180 180 18		
240 230 220 210 210 200 190 190 180 190 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE = 2400 CF5 DESIGN DISCHARGE = 2400 CF5 DESIGN DISCHARGE = 2400 CF5 DESIGN HW ELEVATION = 223.8 FT DASE DISCHARGE = 2440 CF5 DESIGN HW ELEVATION = 224.6 FT 0/ERTOPPING DISCHARGE = 2400 CF5 DASE DISCHARGE = 2400 CF5 150 0/ERTOPPING DISCHARGE = 2400 CF5 150 0/ERTOPPING DISCHARGE = 2400 CF5 150 0/ERTOPPING DISCHARGE = 2400 CF5 150 0/ERTOPPING DISCHARGE = 2440 CF5 150 0/ERTOPPING DISCHARGE = 2440 CF5 150 0/ERTOPPING DISCHARGE = 2440 CF5 150 0/ERTOPPING DISCHARGE = 2400 CF5 0/ERTOPPING DISCHARGE = 2400 CF5 0/ERTOPPING DISCHARGE = 2400 CF5 0/ERTOPPING DISCHARGE = 2400 CF5 0/ERTOPPING DISCHARGE =		
240 230 220 210 200 190 190 180 180 180 170 180 180 170 180 180 170 180 180 170 180 180 170 180 180 180 180 180 180 180 180 180 18		
240 230 220 210 210 200 190 190 180 170 BRIDGE HYDRAULIC DATA DESIGN FREQUENCY 2500 HYDRAULIC DATA DASE FREQUENCY 2500 HYDRAULIC DATA DATE OF SURVEY 2446 CFS DATE OF SURVEY 2500 CFS OVERTOPPING FREQUENCY 150 OVERTOPPING FREQUENCY 2500 CFS 047E OF SURVEY 251-2017 WS.ELEVATION AT DATE OF SURV		
240 230 220 210 210 200 190 190 180 170 BRIDGE HYDRAULIC DATA DESIGN PREQUENCY = 1600 CFS DESIGN FREQUENCY = 25 YRS DESIGN FREQUENCY = 25 YRS DESIGN FREQUENCY = 244 CFS BASE FREQUENCY = 244 CFS BASE FREQUENCY = 100 + 7 YRS OVERTOPPING FREQUENCY = 100 + 7 YRS OVERTOPPING FREQUENCY = 2400 CFS OVERTOPPING FREQUENCY = 100 + 7 YRS OVERTOPPING FREQUENCY = 100 + 7 YRS OVERTOPPING FREQUENCY = 3-1-2017 WS-ELEVATION AT DATE OF SURVEY = 3-1-2017 WS-ELEVATION AT DATE OF SURVEY = 2158 FT 130		
240 230 220 210 210 200 190 190 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE 180 150 00 DESIGN FREQUENCY 150 00 DESIGN FREQUENCY 150 00 DESIGN FREQUENCY 150 00 DESIGN FREQUENCY 150 00 DERTOPPING DISCHARGE 2440 CFS BASE FREQUENCY 150 00 VERTOPPING DISCHARGE 2446 FT 150 00 VERTOPPING DISCHARGE 2400 CFS 00 VERTOPPING ELEVATION 224.4 150 00 VERTOPPING ELEVATION 224.5 140 DATE OF SURVEY		
230 220 210 210 200 190 190 180 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE = 1600 CFS DESIGN FW ELEVATION = 223.8 FT BASE DISCHARGE = 2414 CFS BASE PREQUENCY = 25 YRS DESIGN HW ELEVATION = 223.8 FT BASE DISCHARGE = 2414 CFS BASE FREQUENCY = 100 YRS BASE FREQUENCY = 100 YRS DATE OF SURVEY = 3-1-2017 W.S.ELEVATION = 224.4 FT DATE OF SURVEY = 3-1-2017 W.S.ELEVATION = 224.5 FT 130		240
230 220 210 210 200 190 190 180 180 170 <i>BRIDGE HYDRAULIC DATA</i> <i>DESIGN DISCHARGE</i> = 1600 CFS <i>DESIGN FREQUENCY</i> = 25 YRS <i>DESIGN FREQUENCY</i> = 25 YRS <i>DESIGN HW ELEVATION</i> = 223,8 FT <i>BASE DISCHARGE</i> = 2400 CFS <i>DESIGN HW ELEVATION</i> = 224,6 FT <i>DASE TREQUENCY</i> = 3-1-2017 <i>W.S.ELEVATION</i> = 224,4 FT <i>DATE OF SURVEY</i> = 3-1-2017 <i>W.S.ELEVATION</i> = 215,8 FT 130		
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$		
220 210 200 190 190 180 180 170 180 170 180 170 180 170 180 170 180 170 180 170 180 170 150 05 SIGN FREQUENCY = 25 YRS DE SIGN FREQUENCY = 25 YRS DE SIGN FREQUENCY = 2414 CFS BASE FREQUENCY = 100 YRS BASE FREQUENCY = 100 YRS DATE OF SURVEY = 3-1-2017 W.S.ELEVATION = 224.4 FT DATE OF SURVEY = 3-1-2017 W.S.ELEVATION = 2158 FT 130		230
220 210 210 200 190 190 180 170 BRIDGE HYDRAULIC DATA 180 DESIGN DISCHARGE = 1600 CFS DESIGN FREQUENCY = 253 FT 160 DESIGN HW ELEVATION = 223B FT DESIGN HW ELEVATION = 2246 FT 150 OVERTOPPING FREQUENCY = 100 YRS 150 DVERTOPPING FREQUENCY = 200 CFS 00 YRS DATE OF SURVEY = 3-1-2017 140 W.S.ELEVATION = 224.4 FT 140 DATE OF SURVEY = 3-1-2017 130		
210 200 190 190 190 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE DESIGN DISCHARGE 180 170 BRIDGE HYDRAULIC DATA DESIGN FREQUENCY 25 YR5 DESIGN HW ELEVATION 223.B BASE DISCHARGE 244 CFS DESIGN HW ELEVATION 224.B 150 OVERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 150 0VERTOPPING FREQUENCY 150 0VERTOPPING FREQUENCY 150 0VERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 140 W.S.ELEVATION AT DATE OF SURVEY 215.8 130		220
210 200 190 190 190 180 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE 180 170 BSLIDGE HYDRAULIC DATA DESIGN DISCHARGE 180 170 BASE FREQUENCY 180 DESIGN HW ELEVATION 223.8 BASE DISCHARGE 2414 CFS BASE HW ELEVATION BASE HWELEVATION BASE HELEVATION BASE HELEVATION BASE HELEVATION 0VERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 150 0VERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 140 W.S.ELEVATION AT DATE OF SURVEY 215.8		
210 200 200 190 190 190 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE 160 DESIGN DISCHARGE 160 DESIGN PREQUENCY 223.8 160 DESIGN HW ELEVATION 223.8 BASE DISCHARGE 2414 CFS BASE FREQUENCY BASE FREQUENCY 100 BASE FREQUENCY 150 OVERTOPPING DISCHARGE 244 CFS BASE FREQUENCY 150 OVERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 0VERTOPPING FREQUENCY 140 DATE OF SURVEY 130		010
200 190 190 180 170 180 170 180 170 170 170 170 170 170 170 17		210
200 190 190 180 180 170 BRIDGE HYDRAULIC DATA DESIGN DISCHARGE = 1600 CFS DESIGN FREQUENCY = 25 YRS DESIGN FREQUENCY = 265 YRS DESIGN HW ELEVATION = 223.8 FT BASE DISCHARGE = 2414 CFS BASE FREQUENCY = 100 YRS BASE HW ELEVATION = 224.6 FT OVERTOPPING DISCHARGE = 2400 CFS OVERTOPPING DISCHARGE = 2400 CFS OVERTOPPING FREQUENCY = 100+7- YRS OVERTOPPING ELEVATION = 224.4 FT DATE OF SURVEY = 3-1-2017 W.S.ELEVATION AT DATE OF SURVEY = 215.8 FT 130		
190190180180180170170BRIDGE HYDRAULIC DATADESIGN DISCHARGE170DESIGN DISCHARGE170DESIGN FREQUENCY170BASE DISCHARGE160150150150151152153154155155156157158159150150150151152153154155155155156157158159150150151152153154155155156157158159159150150151152153154155155156157158158159159150150151152153<		200
190190180180180170170BRIDGE HYDRAULIC DATADESIGN DISCHARGEDESIGN FREQUENCY25 YRSDESIGN FREQUENCY25 YRSDESIGN HW ELEVATION223.8 FTBASE DISCHARGE244DESIGN HW ELEVATION224.6 FTOVERTOPPING DISCHARGE0VERTOPPING FREQUENCY1500VERTOPPING FREQUENCY1500VERTOPPING ELEVATION224.4 FTDATE OF SURVEY23.8 FT140140130		
190180180170180170170170170170170170171171172173174175175175176177178179179170171171172173174174175175175175175176177177178179179179179170171172173174174175<		
180180170BRIDGE HYDRAULIC DATADESIGN DISCHARGEDESIGN FREQUENCY25 YRSDESIGN HW ELEVATION223.8 FTBASE DISCHARGE24.4 CFSBASE DISCHARGE24.6 FTOVERTOPPING DISCHARGE24.6 FTOVERTOPPING FREQUENCY150OVERTOPPING FREQUENCY0VERTOPPING FREQUENCY1500VERTOPPING ELEVATION1500VERTOPPING ELEVATION140DATE OF SURVEY140140130		
180180170BRIDGE HYDRAULIC DATADESIGN DISCHARGEDESIGN DISCHARGE160DESIGN KREQUENCY25YRSDESIGN HW ELEVATION223.8FTBASE DISCHARGE2414CFSBASE FREQUENCY100YRSBASE HW ELEVATION224.6FTOVERTOPPING DISCHARGE2400CFSOVERTOPPING FREQUENCY150DATE OF SURVEY24.4FT140MS. ELEVATIONAT DATE OF SURVEY215.8FT130		
BRIDGE HYDRAULIC DATADESIGN DISCHARGEDESIGN DISCHARGEESIGN FREQUENCY= 25YRSDESIGN HW ELEVATION= 223.8FTBASE DISCHARGE= 2414CFSBASE FREQUENCY= 100YRSBASE HW ELEVATION= 224.6FTOVERTOPPING DISCHARGE= 2400CFSOVERTOPPING FREQUENCYISOOVERTOPPING FREQUENCYDATE OF SURVEY= 3-1-2017W.S. ELEVATIONAT DATE OF SURVEY= 215.8FT130		180
BRIDGE HYDRAULIC DATADESIGN DISCHARGEDESIGN DISCHARGEDESIGN FREQUENCY25YRSDESIGN HW ELEVATION223.8FTBASE DISCHARGE24.4CFSBASE HW ELEVATION224.6POVERTOPPING DISCHARGE24.6POVERTOPPING FREQUENCY150OVERTOPPING FREQUENCYOVERTOPPING FREQUENCYDATE OF SURVEY224.4FTDATE OF SURVEY23-1-2017W.S.ELEVATIONAT DATE OF SURVEY215.8FT130		
BRIDGEHYDRAULICDATADESIGNDISCHARGE= 1600CFSDESIGNFREQUENCY= 25YRSDESIGNFREQUENCY= 2414CFSBASEDISCHARGE= 2414CFSBASEFREQUENCY= 100YRSBASEHWELEVATION= 224.6FTOVERTOPPINGDISCHARGE= 2400CFSOVERTOPPINGDISCHARGE= 2400CFSOVERTOPPINGFREQUENCY= 100+/-YRSOVERTOPPINGFREQUENCY= 100+/-YRSOVERTOPPINGFREQUENCY= 3-1-2017140W.S. ELEVATIONATDATEOFSURVEYATDATEOFSURVEY= 215.8FT130		
BRIDGEHYDRAULIC DATADESIGN DISCHARGE= 1600CFSDESIGN FREQUENCY= 25YRSDESIGN HW ELEVATION= 223.8FTBASE DISCHARGE= 2414CFSBASE FREQUENCY= 100YRSBASE HW ELEVATION= 224.6FTOVERTOPPING DISCHARGE= 2400CFSOVERTOPPING FREQUENCY= 100+/-YRSOVERTOPPING ELEVATION= 224.4FTDATE OF SURVEY= 3-1-2017140W.S.ELEVATION= 215.8FT130		
DESIGN DISCHARGE= 1600CFSDESIGN FREQUENCY= 25YRSDESIGN HW ELEVATION= 223.8FTBASE DISCHARGE= 2414CFSBASE FREQUENCY= 100YRSBASE HW ELEVATION= 224.6FTOVERTOPPING DISCHARGE= 2400CFSOVERTOPPING FREQUENCY= 100+/-YRSOVERTOPPING ELEVATION= 224.4FTDATE OF SURVEY= 3-1-2017140W.S. ELEVATIONAT DATE OF SURVEY= 215.8FT130	BRIDGE HYDRAULIC DALA	
DESIGN HW ELEVATION= 223.8FTBASE DISCHARGE= 2414CFSBASE FREQUENCY= 100YRSBASE HW ELEVATION= 224.6FTOVERTOPPING DISCHARGE= 2400CFSOVERTOPPING FREQUENCY= 100+/-YRSOVERTOPPING ELEVATION= 224.4FTDATE OF SURVEY= 3-1-2017140W.S. ELEVATION= 215.8FT130	DESIGN DISCHARGE = 1600 CF DESIGN FREQUENCY = 25 YR	160
BASE DISCIPLINGE = 2414 CFS BASE FREQUENCY = 100 YRS BASE HW ELEVATION = 224.6 FT OVERTOPPING DISCHARGE = 2400 CFS OVERTOPPING FREQUENCY = 100+/- YRS OVERTOPPING ELEVATION = 224.4 FT DATE OF SURVEY = 3-1-2017 W.S. ELEVATION AT DATE OF SURVEY = 215.8 FT 130	DESIGN HW ELEVATION = 223.8 FT $BASE DISCHARGE = 2414 CE$	
BASE HW ELEVATION = 224.6 FT 150 OVERTOPPING DISCHARGE = 2400 CFS OVERTOPPING FREQUENCY = 100+/- YRS OVERTOPPING ELEVATION = 224.4 FT DATE OF SURVEY = 3-1-2017 W.S. ELEVATION = 215.8 FT 130 130	BASE FREQUENCY = 100 YF	rs
OVERTOPPING FREQUENCY = 100+/- YRS OVERTOPPING ELEVATION = 224.4 FT DATE OF SURVEY = 3-1-2017 W.S. ELEVATION AT DATE OF SURVEY = 215.8 FT 130	BASE HW ELEVATION = 224.6 F1 OVERTOPPING DISCHARGE = 2400 CF	-s 150
DATE OF SURVEY = 3-1-2017 W.S. ELEVATION AT DATE OF SURVEY = 215.8 FT 130	OVERTOPPING FREQUENCY = 100+/- YR	rs
DATE OF SURVEY = 3-1-2017 W.S. ELEVATION AT DATE OF SURVEY = 215.8 FT 130	DVLATE OF OUR FILEVALIUN - 224.4 FI	140
$\begin{bmatrix} v.5.LLLVATION \\ AT DATE OF SURVEY = 215.8 FT \\ 130 \end{bmatrix}$	DATE OF SURVEY = 3-1-2017	
130	AT DATE OF SURVEY = 215.8 F7	
		130

INDEX OF SHEETS

TITLE

TITLE SHEET AND INDEX OF SHEETS LIST OF APPLICABLE ROADWAY STANDARD DRAWINGS, LEGEND, GENERAL NOTES, PHASING AND FINAL PAVEMENT MARKING SCHEDULE

TEMPORARY TRAFFIC CONTROL PLAN TEMPORARY TRAFFIC CONTROL PLAN SPECIAL SIGN DESIGN

IP PROJECT:

0

R

ら

7BP

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED DOUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED MOT T MACDONALD 7621 Purfoy Road, Suite 115 Fuquay-Varina, NC 27526 (919) 552-2253 (919) 552-2254 (Fax) www.mottmac.com/americas

GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

LANE AND SHOULDER CLOSURE REQUIREMENTS

A) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 15 FT OF AN OPEN TRAVEL LANE, CLOSE THE NEAREST OPEN SHOULDER USING ROADWAY STANDARD DRAWING NO. 1101.04 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL OR A LANE CLOSURE IS INSTALLED.

TRAFFIC PATTERN ALTERATIONS

B) NOTIFY THE ENGINEER AND LOCAL SCHOOLS & EMS THIRTY (30) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

SIGNING

C) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.

PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL PLANS.

D) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.

COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.

E) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING A PATTERN.

TRAFFIC CONTROL DEVICES

F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

PAVEMENT MARKINGS AND MARKERS

- G) INSTALL PAVEMENT MARKINGS AND MARKERS ON THE FINAL SURFACE ACCORDING TO THE ROADWAY STANDARD DRAWINGS.
- H) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.

MISCELLANEOUS

I) MAINTAIN ACCESS TO ALL RESIDENCES AND BUSINESSES BETWEEN THE CLOSURE POINTS AT ALL TIMES DURING CONSTRUCTION.

TRAFFIC MANAGEMENT PLAN

NCDOT ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" - HIGHWAY DESIGN BRANCH- N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

<u>STD. NO.</u>	TITLE
1101.03	TEMPORARY R
1101.04	TEMPORARY S
1101.11	TRAFFIC CON
1110.01	STATIONARY
1110.02	PORTABLE WO
1135.01	CONES
1145.01	BARRICADES
1205.01	PAVEMENT MA
1205.02	PAVEMENT MA
1205.12	PAVEMENT MA
1250.01	RAISED PAVE
1251.01	RAISED PAVE
1261.01	GUARDRAIL A
1261.02	GUARDRAIL A
1262.01	GUARDRAIL E

- STEP 1: PLACE MESSAGE SIGNS USING ROADWAY STANDARD DRAWING NUMBERS 1101.04, SHEET 1 OF 1,
- INSTALL TYPE III BARRICADES TO CLOSE MONTGOMERY ROAD TO THRU TRAFFIC.
- PLACE FINAL PAVEMENT MARKINGS AND MARKERS.
- STEP 4: REMOVE TYPE III BARRICADES FROM MONTGOMERY ROAD AND REOPEN ROADWAY TO TRAFFIC. REMOVE ALL DETOUR SIGNING.

LICENSE NO. F-0669

FINA	L PA	VEME
DESCRIPTIO	N	
THERMOPLAS PERMANENT	TIC PAVEN RAISED PA	IENT MAR
Μ		ocuSigned by:
MOTT MACDONALD 7621 Purfoy Road, Suite 115 Fuquay-Varina, NC 27526 (919) 552-2253 (919) 552-2254 (Fax) www.mottmac.com/americas	SEAL	AGENES LANGINEES

ANY	TRAI	FFIC		
- ^ ^ !				

PROJECT REFER	ENCE NUMBER	SHEET NO.
17BP.5.R.70	FRANKLIN 28	TMP-2

ROAD CLOSURES HOULDER CLOSURES NTROL DESIGN TABLES WORK ZONE SIGNS ORK ZONE SIGNS

ARKINGS - LINE TYPES AND OFFSETS ARKINGS - TWO-LANE AND MULTI-LANE ROADWAYS ARKINGS - BRIDGES EMENT MARKERS - INSTALLATION SPACING MENT MARKERS - PERMANENT AND TEMPORARY AND BARRIER DELINEATORS - INSTALLATION SPACING AND BARRIER DELINEATORS - TYPES AND MOUNTING GUARDRAIL END DELINEATION

PHASING

1101.11, SHEET 1 OF 4, 1101.03, SHEET 1 OF 9, AND SHEET TMP-3, INSTALL AND COVER DETOUR SIGNING.

STEP 2: USING ROADWAY STANDARD DRAWING NUMBER 1101.03, SHEET 1 OF 9, UNCOVER OFF-SITE DETOUR SIGNING AND

STEP 3: PLACE TRAFFIC ONTO OFF-SITE DETOUR. PERFORM PROPOSED BRIDGE AND ROADWAY CONSTRUCTION.

ENT MARKING SCHEDULE

QUANTITY

RKING LINES (4", 90 MILS) MARKERS (YELLOW & YELLOW) 2,400 LF 8 EA

GENERAL NOTES ROADWAY STANDARD DRAWINGS PHASING PAVEMENT MARKING SCHEDULE

DocuSign Envelope ID: 6106DE7C-83C5-46AE-9B04-B23800A4C5E3

TRAFFIC CONTROL SIGNING AND DEVICES

MONTGOMERY ROAD OFF-SITE DETOUR

SHEET NO.

TMP-4

DocuSign Envelope ID: 6106DE7C-83C5-46AE-9B04-B23800A4C5E3

SIGN	NUMBE	R:SD-	-1		E
	ΤΥΡ	E: ST/	TIONA	RY	(
QU	ANTIT	Y: SEE	E PLAN	IS	SY
SIGN	WIDT	H: 5'-	Ο″		
	HEIGH	T:2'-	6″	_	
ΤΟΤΑ	L ARE	A: 12.	5 Sq.	Ft.	
BORDE	R TYP	E: INS	SET		
	RECES	S:0.4 H·0 6	17" 33"		
	RADI	I: 1.5	5″		
NO.	Z BAR	S:		Γ	/IA1
	LENGTI	Η:			
		USI	Ε ΝΟΤΙ	ES: 1	,2
1.Leg	gend a	nd bo	order	shal.	Lk
nor 2. Bac	i-reti ckarou	ectiv. Ind sh	/e sne nall b	eting De NC	J. GF
ret	troref	lecti	lve sh	eeti	ng.
	IER PO	051110)NS		
	M	0	N	Т	
	7.9	13.1	17.8	22	2
	R	0	Α	D	
	21.7	26	30.2	34.9	

Letter locations are panel edge to lower left corner																		
G	0	M	E	R	Y													
5.8	30.2	35	40.3	44.3	48.2													
																NOD		

PROJECT REFER	ENCE NUMBER	SHEET NO.
17BP.5.R.70	FRANKLIN 28	TMP-5

5.R.70			STATE OF N DIVISION	OR PROPOSED
				ROSION CONTROL
			FRANKL BRIDGE OVER	IN COUNTY NO. 28 ON SR 1210 BILLYS CREEK
PROJEC			$\underbrace{BEGIN \ PROJECT \ I7BP.5.R.70}_{-L- \ STA \ II+25.00} \underbrace{BEGIN \ BRIDO}_{-L- \ Sta. \ I3+}$	SE END BRIDGE 67.88 -L- Sta. 14+25.13L- Sta. 14+25.13
LErosion Control/cadd/340028_EC_tsh				EINT ND BILLS HAT AN
HydroNDDCNFranklin_028NHydraulics)	GRAPHIC SCALE	THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 1, 2016 AND ISSUED BY HE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER RESOURCES.	Prepared in the Office of: FFFE HDR Engineering, Inc. of the Carolinas 555 Fayetteville St, Suite 900 Raleigh, N.C. 27601 N.C.B.E.L.S. License Number: F-0116 Designed by: ALEXANDER D. SNIDER, PE 3064 NAME	Reviewed in the Office of: ROADSIDE ENVIRONMENTAL UNIT I South Wilmington St. Raleigh, NC 27611 2018 STANDARD SPECIFICATIONS Reviewed by: DONALD PEARSON, EI CPESC

•

		STATE	STATE PROJECT REFERENCE NO. SHEET NO. SHEETS
		N.C.	17BP.5.R.70 EC-1
		STATE PROJ.	NO. F. A. PROJ. NO. DESCRIPTION
	EROSIC	N AND	SEDIMENT CONTROL MEASURES
	<u>Std. </u>	<u>Description</u> Temporary	Silt Ditch
	1630.05 1605.01	Temporary Temporary	Diversion TD Silt Fence H
	1606.01 16 22 .01	Special Sed Temporary	liment Control Fence
	1630.02 1633.01	Silt Basin Temporary	Type B Rock Silt Check Type-A
		Temporary Matting an	Rock Silt Check Type-A with ad Polyacrylamide (PAM)
	1633.02	Temporary Wattle // C	Rock Silt Check Type-B
		Wattle // C with Polya	Coir Fiber Wattle acrylamide (PAM)
	1634.01 1634.02	Temporary Temporary	Rock Sediment Dam Type-A
	1635.01 1635.02	Rock Pipe Rock Pipe	e Inlet Sediment Trap Type-A
	1630.04	Stilling Ba	
	1030.06	Special Stil Rock Inle	lling Basin t Sediment Trap:
	1632.01	Туре	
	1632.02	Туре	
17+25.00	1002.00	ıype Skimmer B	CC
		Tiered Ski	immer Basin
		Infiltration	n Basin
			THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION.
VS L			THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED STANDARDS.
			ENVIRONMENTALLY SENSITIVE AREA(S) EXIST ON THIS PROJECT
			Refer To E. C. Special Provisions for Special Considerations.
Roadway Standard I	Drawings		

these plans. 1604.01 Railroad Erosion Control Detail 1605.01 Temporary Silt Fence 1606.01 Special Sediment Control Fence 1607.01 Gravel Construction Entrance 1632.01Rock Inlet Sediment Trap Type A1632.02Rock Inlet Sediment Trap Type B1632.03Rock Inlet Sediment Trap Type C 1632.03Rock Inlet Sediment Trap Type C1633.01Temporary Rock Silt Check Type A1633.02Temporary Rock Silt Check Type B1634.01Temporary Rock Sediment Dam Type A1635.02Temporary Rock Sediment Dam Type B1635.01Rock Pipe Inlet Sediment Trap Type A1635.02Rock Pipe Inlet Sediment Trap Type B1640.01Coir Fiber Baffle1645.01Temporary Stream Crossing 1607.01 Gravel Construction Entrance
1622.01 Temporary Berms and Slope Drains
1630.01 Riser Basin
1630.02 Silt Basin Type B
1630.03 Temporary Silt Ditch
1630.04 Stilling Basin
1630.05 Temporary Diversion
1630.06 Special Stilling Basin
1631.01 Matting Installation

TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)

NOTES:

INSTALL TEMPORARY ROCK SILT CHECK TYPE A IN ACCORDANCE WITH ROADWAY STANDARD DRAWING NO. 1633.01.

USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 4 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.

PROJECT REFERENCE NO SHEET NO. 17BP.5.R.70 EC-2 R/W SHEET NO.

SITE DESCRIPTION

PERIMETER DIKES, SWALES, DITCHES AND

HIGH QUALITY WATER (HQW) ZONES

SLOPES STEEPER THAN 3:1

SLOPES 3:1 OR FLATTER

ALL OTHER AREAS WITH SLOPES FLATTER

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

	STABILIZATION TIME	7/
SLOPES	7 DAYS	NONE
	7 DAYS	NONE
	7 DAYS	IF SLOPE Not ste
	14 DAYS	7 DAYS I Length.
R THAN 4:1	14 DAYS	NONE, EX

PROJECT REFERENCE NO.	SHEET NO.
17BP.5.R.70	EC-3
	-

IMEFRAME EXCEPTIONS

ES ARE IO'OR LESS IN LENGTH AND ARE EEPER THAN 2:1,14 DAYS ARE ALLOWED. FOR SLOPES GREATER THAN 50' IN

(CEPT FOR PERIMETERS AND HQW ZONES.

PROJECT REFERENCE SHEET NO.
17bp.5.r.70 - Franklin 28 EC-05/CONST.04
ROADSIDE ENVIRONMENTAL PROJECT ENGINEER
LEVEL III CERTIFIED BY: ALEXANDER D. SNIDER, PE CERTIFICATION NUMBER: 3064 ISSUED: JANUARY 7, 2019

FINAL EROSION CONTROL FOR CONSTRUCTION SHEET 04

6. Leave compaction hole open. Water thoroughly.

⊢ 2 inch

TREE REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

REFORESTATION MIXTURE, TYPE, SIZE, AND FURNISH SHALL CONFOR 33% LIRIODENDRON TULIPIFERA 33% PLATANUS OCCIDENTALIS 34% BETULA NIGRA

STATE	STAT	SHEET NO.	TOTAL SHEETS	
N.C.		17BP.5.R.70	RF-1	
STAT	te proj. No.	F. A. PROJ. NO.	DESCRIPT	ION

•

REFORESTATION

RM TO THE FOLLOWING:	
TULIP POPLAR	12 in – 18 in BR
	10 in 19 in PD
AMERICAN STCAMORE	12 III = 10 III DK
RIVER BIRCH	12 in – 18 in BR

REFORESTATION DETAIL SHEET

N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT

NOTE: EMBANKMENT COLUMN DOSE NOT INCLUDES BACKFILL FOR UNDERCUT

Approxi borrow paveme

CROSS-SECTION SUMMARY

STATION L	UNCL. EXC. (CU. YD.)	EMBT. (CU. YD.)
12+00.00	0	0
12+50.00	42	1
13+00.00	24	14
13+50.00	8	45
14+00.00	4	52
14+50.00	6	144
15+00.00	8	82
15+50.00	8	53
16+00.00	20	15
16+50.00	37	5

	0 5 10	PROJECT REFERENCE	SHEET NO.
		17BP.5.R.70 – FRANKLIN 28	X–1A
kimate quantities only v excavation, clearing ent will be paid for c	y. Unclassified g and grubbing at the lump su	excavation, fine gradi g, and removal of exist m price for "Grading"	ing, ting ″.

230					
2.30					
230					
230					
230					
2.30					
220					
210					
210					
240					
0.70					
220					
210					
240					
230					
220			 	 	
210					
240					
230					
220					
240					
230					
220					

		0	5 10	PR 17BP.5	OJECT REFER 5.R.70 – FRAN	ENCE IKLIN 28	SHEET NO. X–1
0 0	00	100	110	120	120	140	150
	90	100				140	
							230
							220
							210
							0.40
							270
							220
							210
							240
							230
							220
							210
							240
							230
							220
							240
							000
80	90	100	110	120	130	140	150

15:01 PN 340028

		0	5 10	PRC	DJECT REFERENC	E	SHEET NO.
				17BP.5	R.70 – FRANKLIN	1 28	X–2
80	90	100	110	120	130	140	150
							230
							0.00
							220
							210
							230
							220
							210
							230
							220
							210
							230
							220
			<u> </u>				
							210
							230
							220
					/		
							210
							2.10
							230
							220
							210
80	90	100	110	120	130	140	150

		0 • • • • • • • • • • • • • • • • • • •	5 10	PRC	DJECT REFERE		SHEET NO.
00	00	100	110	120	120		160
80	90			120	130	140	
							<i>010</i>
							230
							220
							210
			110	100	120		160
80	70			120	130	140	UDU UCI

_	(-)3.4143% <u>(</u> -)1.2367%	
	PVI STA.12+70.00 EL.= 226.98 VC = 140'	
_	(-)1.2367% (+)1.9625%	
	PVI STA.15+70.00 EL.= 223.27 VC = 160'	
	GRADE DATA -L-	

HYDRAULIC DATA:	
DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD DESIGN HIGH WATER ELEVATION DRAINAGE AREA BASE DISCHARGE (Q 100) BASE HIGH WATER ELEVATION	= 1600 CFS = 25 YEAR = 223.8 = 6.3 SQ.MI. = 2414 CFS = 224.6
OVERTOPPING FLOOD DATA:	
OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD OVERTOPPING FLOOD ELEVATION ** OVERTOPPING OCCURS AT ROADW SAG AT STA.15+52.00 -L- AT ROADWAY SUPER HIGH (LEFT) SIDE	= 2400 = 100± YEAR = 224.4 * * AY

BM #8 BENCHTIE SET IN 16" OAK, STA. 13+82.83 -L-, 97.91' LT., EL. 220.97

DRAWN BY:	S.D. COOPER		DATE:	6-21
CHECKED BY:	B.S. COX		DATE:	6-21
DESIGN ENGIN	EER OF RECORD:	B.S. COX	DATE: _	6-21

.017
т т В
VFY -I-
1
<u>т т т</u>
SS II RAP
YP.)
ONS.

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

FOR EROSION CONTROL MEASURES. SEE EROSION CONTROL PLANS.

THE BRIDGE SHALL BE REMOVED WITH CONTAINMENT MEASURES IN PLACE TO PREVENT COMPONENTS OF THE BRIDGE DECK FROM DROPPING INTO THE STREAM. THE ASPHALT WEARING SURFACE SHALL BE REMOVED FIRST FOLLOWED BY REMOVAL OF THE RAILS, DECKING, GIRDERS AND FINALLY THE END BENT ABUTMENTS AND THE INTERIOR BENT CAP AND PILES. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS AND CONTAINMENT MEASURES FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

REMOVE EXISTING END BENTS IN ENTIRETY. INTERIOR TIMBER PILES SHALL BE COMPLETELY REMOVED OR BROKEN ONE FOOT BELOW THE MUD LINE.

NO DECK DRAINS ARE ALLOWED ON THE BRIDGE.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT. LEFT AND 25 FT. RIGHT OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTS OF 2 CONTINUOUS SPANS @ 20'-3". THE SUPERSTRUCTURE CONSISTS OF TIMBER DECK ON CONTINUOUS STEEL I-BEAMS AND HAS A CLEAR ROADWAY WIDTH OF 19'-2". THE END BENTS AND INTERIOR BENT CONSIST OF TIMBER CAPS ON TIMBER PILES. THE EXISTING STRUCTURE, WHICH IS LOCATED AT THE SITE OF THE PROPOSED STRUCTURE, SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THE LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES. SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, PRESTRESSED CONCRETE END BENT CAPS MAY BE SUBSTITUTED IN PLACE OF THE CAST-IN-PLACE CAPS. THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER TO RECEIVE REVISED PLANS AND DETAILS FROM THE STRUCTURES MANAGEMENT UNIT. THE REDESIGN AND ANY ADDITIONAL MATERIALS NEEDED WILL BE AT NO ADDITIONAL COST TO THE CONTRACTOR.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 13+96.50 -L-."

OF MA	TER	IAL						
DRIVING MENT SETUP HP 12 X 53 EL PILES	HP 12 STEEL	X 53 PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0" PREST CON CORED	X 1'-9″ TRESSED CRETE) SLABS
EA	NO.	LF	LF	TON	SY	LS	NO.	LF
			110.25			LS	10	550.00
5	5	100		105	120			
5	5	100		95	105			
10	10	200	110.25	200	225	LS	10	550.00

PLANS PREPARED

DOCUME UNLESS /

	ſ	PROJEC	CT NO.	17B	P.5.R	.70
	-	F	RANKL	IN	CO	UNTY
		STATI	ON: <u>13</u>	5+96	.50 -	L
		SHEET 2	OF 2			
		DEPA	STATE O	OF NORTH CAR OF TRAI RALEIGH	OLINA NSPORTA	TION
BY:		G	ENERA	L DF	RAWIN	١G
RS IATES	Docusion addy CAROL	F	OR BRID	GE ON GOMER TUUYS	SR 121 Y RD) CREEK	0
ive	SEAL 1 1268	BE	TWEEN S	R 1211	AND US	5 1
(Fax)	THE COLUMN	27'-10"	CLEAR R	OADWA	Y - 90	• SKEW
C-2521	7/9/2021	NO. BY:	REVISIO	DNS D. BY:	DATE:	SHEET NO. S-2
ENT NOT CONSIDE	RED FINAL COMPLETED	1	3 4	8		total sheets 15

		LOAD AN	D RE	SIST	fance	E FAC	CTOR	RAT	ING	(LRF	D) SI	JMMA	ry f	OR I	PRES	TRES	SSED	CON	CRET	E GI	RDEF	? S	
	STRENGTH I LIMIT STATE												SE	SERVICE III LIMIT STATE									
										MOMENT					SHEAR						MOMENT		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)
		HL-93(Inv)	N/A	1	1.055		1.75	0.275	1.23	55′	EL	27	0.523	1.23	55′	EL	5.4	0.80	0.275	1.05	55′	EL	27
DESTGN		HL-93(0pr)	N/A		1.591		1.35	0.275	1.59	55′	EL	27	0.523	1.59	55′	EL	5.4	N/A					
LOAD		HS-20(Inv)	36.000	2	1.322	47.585	1.75	0.275	1.54	55′	EL	27	0.523	1.47	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27
RAIING		HS-20(0pr)	36.000		1.9	68.396	1.35	0.275	1.99	55′	EL	27	0.523	1.9	55′	EL	5.4	N/A					
		SNSH	13.500		2.776	37.476	1.4	0.275	4.04	55′	EL	27	0.523	4.17	55′	EL	5.4	0.80	0.275	2.78	55′	EL	27
		SNGARBS2	20.000		2.155	43.095	1.4	0.275	3.14	55′	EL	27	0.523	3.02	55′	EL	5.4	0.80	0.275	2.15	55′	EL	27
		SNAGRIS2	22.000		2.079	45.734	1.4	0.275	3.03	55′	EL	27	0.523	2.83	55′	EL	5.4	0.80	0.275	2.08	55′	EL	27
		SNCOTTS3	27.250		1.384	37.708	1.4	0.275	2.01	55′	EL	27	0.523	2.09	55′	EL	5.4	0.80	0.275	1.38	55′	EL	27
	S<	SNAGGRS4	34.925		1.189	41.527	1.4	0.275	1.73	55′	EL	27	0.523	1.77	55′	EL	5.4	0.80	0.275	1.19	55′	EL	27
		SNS5A	35.550		1.16	41.255	1.4	0.275	1.69	55′	EL	27	0.523	1.82	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27
		SNS6A	39.950		1.079	43.102	1.4	0.275	1.57	55′	EL	27	0.523	1.68	55′	EL	5.4	0.80	0.275	1.08	55′	EL	27
LEGAL		SNS7B	42.000		1.028	43.175	1.4	0.275	1.5	55′	EL	27	0.523	1.67	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27
LOAD		TNAGRIT3	33.000		1.32	43.556	1.4	0.275	1.92	55′	EL	27	0.523	1.98	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27
RATING		TNT4A	33.075		1.33	43.979	1.4	0.275	1.94	55′	EL	27	0.523	1.91	55′	EL	5.4	0.80	0.275	1.33	55′	EL	27
		TNT6A	41.600		1.101	45.811	1.4	0.275	1.6	55′	EL	27	0.523	1.83	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27
	ST	TNT7A	42.000		1.114	46.804	1.4	0.275	1.62	55′	EL	27	0.523	1.71	55′	EL	5.4	0.80	0.275	1.11	55′	EL	27
		TNT7B	42.000		1.163	48.848	1.4	0.275	1.69	55′	EL	27	0.523	1.62	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27
		TNAGRIT4	43.000		1.101	47.33	1.4	0.275	1.6	55′	EL	27	0.523	1.56	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27
		TNAGT5A	45.000		1.031	46.405	1.4	0.275	1.5	55′	EL	27	0.523	1.58	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27
		TNAGT5B	45.000	3	1.013	45.582	1.4	0.275	1.47	55′	EL	27	0.523	1.48	55′	EL	5.4	0.80	0.275	1.01	55′	EL	27

2021				
3/2	DRAWN BY:	S.D. COOPER		DATE: 6-21
3/	CHECKED BY:	B.S. COX		DATE: 6-21
	DESIGN ENGIN	EER OF RECORD:	B.S. COX	DATE: 6-21

5:31

 \sim

PLANS PREPARED

DOCUMEN UNLESS AL

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	γ_{DW}
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MBER

ĭ

COMMENT

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN. DISTANCE FROM LEFT END OF SPAN IS MEASURED FROM 🕻 BEARING.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
$\sqrt{3}$ LEGAL LOAD RATING **
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER – EXTERIOR RIGHT GIRDER

		PROJEC <u>F</u> STATIC	CT NO. RANK DN: 1	<u>178</u> LIN 3+96	<u>P.5.R</u> co .50 -	<u>.70</u> UNTY L-
		DEPA	STATE RTMENT	OF NORTH CAR	OLINA NSPORTA	TION
BY: S ATES	Doorte Rolad by CAROL BULSY OFESSION 80307 OSE9D89ETAL I 1268	LR 55′	FR SI CORE 90	UMMA DSL SK	RY F AB UI EW	OR NIT
ax) jr.com -2521	7/9/2021		REVIS			SHEET NO.
NT NOT CON LL SIGNATU	NSIDERED FINAL IRES COMPLETED	1		3 4		TOTAL SHEETS 15

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

PLANS PREPARED

DOCUM UNLESS

		PR ST	OJEC F	CT NO. RANK DN: 1	 	<u>17B</u> IN +96,	<u>P.5.R</u> co .50 -	<u>.70</u> UNTY L-
		SH	EET 1 (OF 4				
D BY: ERS CIATES	Designation of AROL IN THE SECOND	P	depa 'RES	SUPE 3'-(STRES	rs RS Cr	NORTH CAR TRAN RALEIGH TRUC X 1 ED	NSPORTA TURE 1-9"	TION RETE
rive	SEAL I 1268		С	ORED	S	SLAE	3 UNI	T
	MGINEE .		-	9	0 °			
(Fax) ngr.com	TATE SY S. CONT			REVIS	SION		_ 11	SHEET NO.
C-2521	7/9/2021	NO.	BY:	DATE:	NO.	BY:	DATE:	S-4
ENT NOT CON ALL SIGNATU	SIDERED FINAL RES COMPLETED	1 2			3 4			total sheets 15

18'-4"	▶ ◄
E GROUTED ESS DETAILS (TYP.)	10-#5 B14 IN VERTICAL CONCRETE BARRIER RAIL
UTTERLINE _	#5 S3 & #5 S4
	4"
(TYP.EA.SLAB UNIT) (T)	YP.) (TYP.
=======================================	
	<u> L</u>
<u> </u> <u>1′−9″</u>	
SPLICE	
0.6"Ø L.R. TRANSVERSE	
N $2^{1}/_{2}^{"} \varnothing$ HOLE (TYP.)	
GUTTERLINE	
	#4 S2 +4 S2 VERTICAL CONCRETE
MĂT'Ĺ. IN RAIL (TYP.)	BARRIER RAIL
PAIRS (SPACED AS SHOWN IN DETAIL ``A'') (TYP.EA.	UNIT)
I 3 (SPACED AS SHOWN IN DETAIL ``A'')(TYP.EA.EXT.U PACED TO MATCH S3 IN VERTICAL CONCRETE BARRIE	NIT) R RAIL)
→ <	27'-6"
55′-0″	
	PLANS PREPARED
ILAN UF SFAN A	E ASSOC
	5640 Dillard Di Suite 200 Cary, NC 27518 (919) 852-0468 (919) 852-0598
	LICENSURE NO.

CORED	SLABS	S REQ	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
55' UNIT			
EXTERIOR C.S.	2	55′-0″	110'-0"
INTERIOR C.S.	8	55'-0"	440'-0"
TOTAL	10		550'-0"

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0"× 1'-9"
55' CORED SLAB UNIT	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1 ∕₂″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	3∕8″ ↓
FINAL CAMBER	1 ∕8″ ♦
** INCLUDES FUTURE WEARING SURF	FACE

BILL OF MATERIAL FOR ONE 55' CORED SLAB UNIT								
	EXTERIOR UNIT INTERIOR UNIT							
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	
B7	4	#4	STR	28'-3″	75	28′-3″	75	
S1	8	#5	3	4'-3"	35	4'-3"	35	
S2	114	#4	3	5′-4″	406	5′-4″	406	
* S3	64	#5	1	5′-7″	373			
REINFO	DRCING	STEEL	LBS	5.	516		516	
✤ EPOX	Y COATE	ED						
REINFORCING STEEL LBS. 373								
6500 P.S.I. CONCRETE CU. YDS. 7.8 7.8					7.8			
0.6″Ø	L.R. STR	ANDS	Nc).	19		19	

CONCRETE RELEASE STRENGTH			
UNIT	PSI		
55' UNITS	4900		

GRADE 270 STRANDS			
	0.6″ØL.R.		
AREA (SQUARE INCHES)	0.217		
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600		
APPLIED PRESTRESS (LBS.PER STRAND)	43,950		

PLANS PREPARED

DOCUMEN UNLESS AL

		PROJEC	ст no. <u>RANKL</u> ом. 13	<u>178</u> _IN 3+96.	<u>P.5.R</u> co .50 -	<u>.70</u> UNTY L-
	·					
		SHEET 3	OF 4			
		DEPA		OF NORTH CAR	OLINA NSPORTA	TION
BY:			SUPEF	SIRUC	/ I UKE	
RS ATES Ive	BOODER DOOR ALE SSION BOULTONESDOOR ALE SEAL I 1268	PRES C	3'-0 STRESS ORED	″X1 SED SLAE	'-9" CONCI 3 UNI	RETE T
Fax)	THE STAR COLUMN		90)° SKE	W	
gr.com	7/9/2021		REVISI	IONS		SHEET NO.
2-2521	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	NO. BY:	DATE: N	IO. BY:	DATE:	S-6
NT NOT COI	NSIDERED FINAL JRES COMPLETED	1		3 1,		TOTAL SHEETS 15

BII	L OF MATERIAL FOR VERTIO	CAL CONCR	ETE I	BARR:	IER RA	11/
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WE
	55' UNIT					
₩ B14	40	40	#5	STR	27'-1″	
* S4	128	128	#5	2	7'-2″	
★ EPOX	Y COATED REINFORCING STEEL			LB		
CLASS	AA CONCRETE			CY		
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LF		11

GUTTERLINE ASP	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
55' UNITS	15⁄8″	3′-75⁄8″

NOTES:

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS. RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS. THE $2^{1}/2^{\prime\prime}$ Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT. THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS. WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS.AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS. LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED. ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED. PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS. APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS. GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH. FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED. THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS. THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION. THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE. THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK. THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS. PROJECT NO. 178P.5.R.70 FRANKLIN COUNTY 13+96.50 -L-STATION: SHEET 4 OF 4 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE 3'-0" X 1'-9" PRESTRESSED CONCRETE ty Stession 39D9944E SEAL 1 1268 CORED SLAB UNIT

90° SKEW

NO. BY:

REVISIONS

DATE:

BY:

SHEET NO.

S-7

TOTAL SHEETS

15

DATE:

NGINEE 7/9/2021 **DOCUMENT NOT CONSIDERED FINAL**

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ "HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE $1\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

	PROJECT NO. <u>17BP.5.R</u> FRANKLIN CC STATION: <u>13+96.50</u>	<u>.70</u> UNTY L-
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTA RALEIGH SUPERSTRUCTURE	TION
IATES (Fgx)	GUARDRAIL ANCHOF DETAILS FOR VERTICAL CONCRE BARRIER RAIL	RAGE ETE
igr.com C-2521 7/9/2021	REVISIONS NO. BY: DATE: NO. BY: DATE:	SHEET NO. S-8
ENT NOT CONSIDERED FINAL ALL SIGNATURES COMPLETED	1 3 2 4	total sheets 15

NOTES:

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS. THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED. FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

TOP OF PILE ELEVATIONS				
	222.02			
2	221.86			
3	221.69			
4	221.53			
5	221.36			

		PROJE(CT NO.	<u>176</u> i tni	<u>3P.5.R</u>	<u>.70</u>
STATION: 13+96.50 -L-						
		SHEET 1	OF 4			
-	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
BY:			20B	SIRUC	IUKE	
RS ATES Ive	BUTTO SEAL BUTTO SEAL BUTTO SEAL	END BENT 1				
Fax) ar.com	The incluster of the interview of the in			TONC		
C-2521	7/9/2021	NO. BY:	REVIS		DATF:	SHEET NU. S-9
	SIDERED FINAL	1		3		TOTAL SHEETS
	RES COMPLETED	え		A		15

NOTES:

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS. THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED. FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

TOP OF PILE ELEVATIONS				
	221.35			
2	221.19			
3	221.02			
4	220.86			
5	220.69			

		PF	ROJE(F	T NO. RANK	_ 	<u>178</u> <u>IN</u> +96	<u>P.5.R</u> co	<u>.70</u> UNTY
		21			<u> </u>	<u>' JU</u> ,		<u> </u>
		SF	HEET 2	OF 4				
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						TION	
BY:				SUD	SI		URE	
RS IATES 'ive	BOTHER BOARD	END BENT 2						
Fax)	THE STAY S. COLUMN							
gr.com	7/9/2021			REVIS	101	NS		SHEET NO.
2-2521		N0.	BY:	DATE:	NO.	BY:	DATE:	S-10
INT NOT CO	NSIDERED FINAL JRES COMPLETED	1 2			3 A			total sheets 15

٩R	TYPES ———		BI	LL O	F MA	ATERIA	L
		FOR ONE END RENT					
		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
K.	4'/2'' + 2'-5'' + 4'/2''' + 4'/2''' + 4'/2''' + 4'/2''' + 4'/2'''' + 4'/2'''''' + 4'/2'''''''''''''''''''''''''''''''''''	B1	8	#9	1	38′-0″	1034
_ ″		B2	28	#4	STR	19′-1″	357
-3"	НК.	B3	9	#4	STR	2'-5″	15
		D1	20	#6	STR	1'-6″	45
	<i>~</i> ──1′−3″ LAP						
		H1	40	#4	2	9'-4"	249
	$\langle \gamma \rangle$	1/ 1	10	# 1		0/ 11//	71
			10	4		2 -11	51
		<u>S1</u>	46	#4	र	10'-5"	320
	$\left(\begin{array}{c} \overline{5} \end{array}\right)$	S2	46	#4	4	3'-2"	97
		S3	20	#4	5	6'-6"	87
		V1	52	#4	STR	6'-2″	214
	<u>1′-8″Ø</u>						
		(FOR	ONE E	NG STE ND BEN	EL IT)		2449 LB
		CLASS			E BREA	AKDOWN	
			IFUR (JNE ENI	U BEN	1)	
		POUR	1 C. 0	AP,LOW F WING	VER PA SS & (RT COLLARS	17 . 9 CY
ONS	ARE OUT TO OUT.	POUR	2 U	PPER P	ART C	۶F	2 . 1 CY
	END BENT 2		W	INGS			
	HP 12 X 53 STEEL PILES						
	NO: 5 LF = 100	TOTAL	_ CLAS	SS A C	ONCRE	TE	20.0 CY
	PILE DRIVING EQUIPMENT						
	HP 12 X 53 STEEL PILES						
	NO: 5						
		I					

		PROJEC	CT NO.	<u> 17B</u>	P.5.R	.70		
		F	RANK	LIN	CO	UNTY		
		STATION: 13+96.50 -L-						
		SHEET 4	OF 4					
г		DEPA	stat RTMENT SLIB	E OF NORTH CAR OF TRAN RALEIGH	OLINA NSPORTA TURE	TION		
BY:			500	511001	UNL			
ATES	Docubert to CARO/ HOLTON SESSION 803D DODDOD9944E	E	END E Di	BENT ETAIL	1 & 2 _S	2		
ve Fax)	I I268							
gr.com -2521	7/9/2021		REVIS	SIONS	DATE	SHEET NO. S-12		
NT NOT CON LL SIGNATUI	SIDERED FINAL RES COMPLETED	1	DATE	3 4		total sheets 15		

FRANKLIN COUNTY STATION: 13+96.50 -L-								
_		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						
Y: ES *	BUTSU GAROL SUTSU GALSSION SEAL I 1268	RIP RAP DETAILS						
com	7/9/2021			REVI	SION	S		SHEET NO.
<u> </u>		NO.	BY:	DATE:	NO. ସ୍ଥ	BY:	DATE:	TOTAL
F NOT CON SIGNATU	ISIDERED FINAL RES COMPLETED	∎ 2			৩ ব্রু			SHEETS 15

ESTIMATED QUANTITIES					
BRIDGE @ STA.13+96.50 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
END BENT 1	105	120			
END BENT 2	95	105			

PROJECT NO. 17BP.5.R.70

FRANKLIN

NOTES:

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4"Ø DRAINAGE PIPE, AND SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS. GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB. FOR THE 4"Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS. AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL

BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.

CURB DETAILS

BILL OF MATERIAL						
А	PPR	OACH	SLA	ΒΑΤΕ	B 1	
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
* A1	13	#4	STR	28'-10″	250	
A2	13	#4	STR	28'-10″	250	
米 B1	58	#5	STR	11'-2″	676	
B2	58	#6	STR	11'-8″	1016	
REINF	ORCIN	G STEE	L	LB	1266	
* EPC REI	XY CC	ATED ING ST	EEL	LB	926	
CLASS	AA C	ONCRET	Έ	CY	16.7	
А	PPR	DACH	SLA	ΒΑΤΕ	B 2	
BAR	N0.	SIZE	TYPE	LENGTH	WEIGHT	
* A1	13	#4	STR	28′-10″	250	
A2	13	#4	STR	28′-10″	250	
米 B1	58	#5	STR	11'-2″	676	
B2	58	#6	STR	11'-8″	1016	
REINF	ORCIN	G STEE	L	LB	1266	
* EPC REI	XY CC	LB	926			
CLASS	AA C	ONCRET	E	CY	16.7	

SPLICE CHART					
BAR SIZE	EPOXY COATED	UNCOATED			
#4	1'-11″	1'-7"			
#5	2′-5″	2'-0″			
#6	3′-7″	2′-5″			

DRAWN BY:_

DOCUME **UNLESS A**

			T NO.	<u>178</u>	P.5.R	<u>.70</u>
STATION: 13+96.50 -L-						UNTY L-
		SHEET 2	OF 2			
_		DEPA	STATE RTMENT	OF NORTH CAR	OLINA NSPORTA	TION
BY:						
RS ATES	BUT STATES BUT AT A STATES	BI	RIDGE SLAB	E APF DFT	PROAC	CH
ive Fax)	II268					
gr.com C-2521	7/9/2021		REVIS	IONS	DATE	SHEET NO.
INT NOT CON	NSIDERED FINAL	no. Bi: 1 2	DATE:	™. BY: 3 4	DATE:	TOTAL SHEETS 15

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT:

ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER. DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE

AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION. HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4"Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY. AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING. GALVANIZING. OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

70 X. ら 7BP. PROIEC

340028 5 REFERENCE

CONTENTS	
<u>SHEET NO.</u>	DESCRIPTION
I.	TITLE SHEET
2	LEGEND
3	BORING LOCATION PLAN
4	PROFILE
5-6	CROSS SECTIONS
7-8	BORE LOGS

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY FRANKLIN

PROJECT DESCRIPTION BRIDGE NO. 28 ON SR 1210 (MONTGOMERY RD.) OVER BILLY'S CREEK

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	SF-340028	1	8

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOLI TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6860. THE SUBSIFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CALITORIED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPNION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONSTRUCTIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OF FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FOM THE ACTUAL CONDENSATION.

NOTES:

- TES: THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT. BY HAVING REDUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

TRIGON

GOODNIGHT, D.J.

INVESTIGATED BY _____GOODNIGHT, D.J.

DRAWN BY <u>*HILL, M.J.*</u>

CHECKED BY HUNSBERGER, W.S.

SUBMITTED BY ______ FALCON ENG.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM DIS66), SOIL CLASSIFICATION IS BASED ON THE AGATTO SYSTEM, BASIT DESCRIPTIONS GENERALLY INCLUDE THE COLL OWING.	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA.
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF. FOR SULTY CAR, MOIST WITH INTERBEDED FINE SAND LAVERS, HIGHLY PLASTIC.A-7-6	ANGULARITY OF GRAINS THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	AMENALEUS - APPLIED TO HOUCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. <u>ARGULACEOUS</u> - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE.SLATE.ETC.
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED. MINERALOGICAL COMPOSITION	WEATTREED VIEW TURNELLAIN MATERIAL THAT WUULD YTELD SPIN VALUES > ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (< 35% PASSING *200) (<> 35% PASSING *200) ORGANIC MATERIALS GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	CRYSTALLINE ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE, CMEISS, GABBRO, SCHIST, ETC.	SURFACE. <u>CALCAREOUS (CALC.)</u> - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-5 A-3 A-6, A-7 SYMBOL 000000000000000000000000000000000000	COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LL < 31 MORDATIVE COMPRESSIBLE LL < 31 MORDATIVE COMPRESSIBLE	NON-CRYSTALLINE CONTACT AND A CONTRACT AND A CONTRA	$\underline{\text{COLLUVIUM}}$ - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
<u>робоодооод</u>	HIGHLY COMPRESSIBLE LL > 50 HIGHLY COMPRESSIBLE LL > 50 PERCENTAGE OF MATERIAL	CONSTRE FEHIN SCHEMENTS CEMENTED LINE TO THE CONSTRE FEHING SCHEMENTED LINE TO THE CONSTRE FEHING SCHEMENTED LINE STORE, SANDSTONE, CEMENTED CONSTRE FEHING SCHEMENTED LINE STORE, SANDSTONE, CEMENTED LINE STORE SCHEMENTED LINE SCHEMENTED L	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*40 30 MX 50 MX 51 MN *200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN	ORGANIC MATERIAL OR SILT - CLAY SOILS SOILS OTHER MATERIAL SOILS OF ORGANIC ANTER A	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIRE - H HADLEH BUDT OF TOMEDUS ROCK THE CUTS HERUSS THE STRUCTURE OF HUJHLENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40 LL – – 48 MX 41 MN	ITRALE OF UNGANIL MATTER 2 -3/ 3 -5/ ITRALE 1 -10/ LITTLE ROADERATER 3 -5/ 5 12/ LITTLE 10 -20% MODERATELY ORGANIC 5 -10% 12 -20% SOME 20 -35%	HAMMER IF CHYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. (V SIL) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF	HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 18 MX 18 MX 11 MN 11 MN 18 MX 18 MX 11 MN 11 MN 11 MN 11 MN 11 MN 000000000000000000000000000000000000	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE GROUND WATER	OF A CRYSTALLINE NATURE. SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
USUAL TYPES STONE FRAGS. OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND SOLS SOLS OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND SOLS SOLS	✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ✓ STATIC WATER LEVEL AFTER 24 HOURS	(SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOBO COCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	SIDES RELATIVE TO UNE ANUTHER FARALLEL TO THE FRACTORE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MRILHIRLS SANU Constraints GEN.RATING as suprova EXCELLENT TO GOOD FAIR TO POOR FAIR TO poor POOR	Very PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	MUDERATE SIGNIFICANT PUNTIONS OF HOUK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	<u>ILLUAI</u> - MULK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
PI 0F A-7-5 SUBGROUP IS ≤ LL - 30 ;PI 0F A-7-6 SUBGROUP IS > LL - 30 CONSISTENCY OD DENCENFCC		WITH FRESH ROCK. MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED COMPACTNESS OR PENETRATION RESISTENCE COMPRESSIVE STRENGTH		MOD. SEV. AND CAN BE EXCAMPTED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL</i>	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
GENERALLY VERY LOOSE (4 GENERALLY VERY LOOSE (4 GENERA	U WITH SOIL DESCRIPTION → OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAQLINIZED TO SOME EXTENT SOME ERACINE OR STRONG PORCHIGUELY REMAIN	ITS LATERAL EXTENT. L <u>ENS</u> - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR LUUSE 4 10 10 MATERIAL MEDIUM DENSE 10 10 30 N/A MATERIAL DENSE 30 10 50 10 10	ARTIFICIAL FILL (AF) OTHER THAN ROADWAY FMBANKMENT AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT IN VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERY DENSE > 50 VERY SOFT < 2 < 0.25	INFERRED SOIL BOUNDARY	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DECREE THAT ONLY MINOR VESTICES OF DRICINGL ROCK EABLIC REMAIN IN FEFESTED WITH VELOS FOR MULTING	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
GENERALLY SUF1 2 10 4 0.25 10 0.5 SILT-CLAY MEDIUM STIFF 4 T0 8 0.5 T0 1.0 MATERIAL STIFF 8 T0 15 1 T0 2 (COHESIVE) VERY STIFF 15 T0 30 2 T0 4	TITE INFERRED ROCK LINE MUCH MONITORING WELL TEST BORING WITH CORE VITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
		ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 DEFNING (MM) 4.75 2.00 0.42 0.25 0.075 0.073	UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	ROCK. <u>SILL</u> - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT. THAT HAS BEEN EMPLACED PARALLEL TO
BOULDER COBBLE GRAVEL COARSE FINE SLLT CLAY	SHALLOW UNDERCUT UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF ACCEPTABLE DEGRADABLE ROCK EMBANKMENT OR BACKFILL	HARD CAN BE SCHAICHED BY KNIFE OH PICK UNLY WITH DIFFICULIY. HARD HAMMEN BLOWS REQUIRED TO DETACH HAND SPECIMEN. MODERATELY CAN BE SCHAICHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	THE BEDDING OR SCHISTOSITY OF THE INTRUGED ROCKS. <u>SLICKENSIDE</u> - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
CCDR.1 CCDR.2 CCR.2 <	ABBRE VIATIONS AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3 SOIL MOISTURE - CORRELATION OF TERMS	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY γ - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC 2 - DRY UNIT WEIGHT	MEDIUM CAN BE GROUVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A CEOLOGIST'S PICK.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOLL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
SOIL MOISTURE SCALE FIELD MOISTURE (ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT, SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT,) FROM BELOW THE GROUND WATER TABLE	6 - VOID RATIO SD SAND, SANDY SS SPLIT SPOON F - FINE SL SILT, SILTY ST SHELBY TUBE	PIELES CAN BE BROKEN BY FINGER PRESSURE. VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORF IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED BEADILY BY	STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC - WET - (W) SEMISOLID; REQUIRES DRYING TO	FUSS FUSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRACMENTS // - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
	HL - HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	N: 873434.IE: 2173860.5 STA. II+95 OFFSFI: I4' I TI - FI EVATION: 229 01 FFFT
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: CME-45C CLAY BITS IX AUTOMATIC MANUAL	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.08 - 0.16 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	X CME-55	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	FIAD - FILLED IMMEDIATELY AFTER DRILLING
PLASTICITY		INDUKATIUN	
PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW	U CME-5500 HARD FACED FINGER BITS	FRIABLE	
SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM HIGHT PLASTIC 26.0P MORE LICU	VANE SHEAR TEST CASING W/ ADVANCER HAND TOOLS:	UENILE BLUW BY HAMMER DISINIEGRATES SAMPLE. MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE:	
COLOR		BREAKS EASILY WHEN HIT WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).		INDURATED DIFFICULT TO BREAK WITH HAMMER.	
		SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14

project reference no. SF-340028

SHEET NO.

					PROJE	CT REFERENCE	5HEEL NO. 4						
				: : :	ROADWA	Y DESIGN		HYDRAULICS ENGINEER					
÷				j	ENG			LINGHALLEN					
-		GRA											
	51A.	10+3	50.00										
	EL ≓	= 224	4.84′			NOT USE FOR	R/W A	CQUISITION					
					HATCH MOTT MAC	DONALD I& E, LLC NO. F-0669		ICA ENGINEERING LICENSE NO. F-0258					
	01	07				ANT NOT C		ERED FINAL					
900			NIC		VERTICA	L SCALE	25/						
200	1015101	nE UNGA					·						
47	24												
TY	SAND	AND S	SAND										
								240					
								i i					
							-						
								230					
-													
	-												
				:									
-													
				, 									
-													
	÷			• • •	• •			200					
				; :				200					
;				,									
	÷			:									
								190					
;				, ,									
				:									
				, , ,	, , ,								
-													
						_							
	1	6			1	7							

=

=

			0 2;	5 5	proj. ri SF -	FERENCE NO	D. SH	EET NO. 5
4	104	55	0 5	5 6	0 6	5 7	0 7	5
								270
	;							265
		· · · · ·						250
		· · · · ·						
		· · · · · · · · · · · · · · · · · · ·						250
								245
	· · ·							
								235
	· · ·							
								230
								225
	1	3						
. . 					· <u> </u>			220
								2/5
	•							210
								205
								200
····	DOSE	ECTION	41.020	DENT		D / 11/27		/95
L C DGR FRO	APHIC M MOI	DATA F T MACI	ROM EI	DENI LECTRO DATED	NIC FIL	ES H 2017.		190
STR THE	ATIGRA BORIN	PHY IS GS WITI	DRAWN H BOTH					
О́Λ Е₩:	TO THE	CROSS REES	SECTIC	N.				185
	- + + +	· · · · · · · · · · · · · · · · · · ·						
	0 4	5 5	0 5	5 4	0 4	5 7	0 7	5
	- 7			. 0	0	- /	- //	-

	0 2:5 5	PROJ. REFERENCE NO. SF-340028	SHEET NO.
40 45	50 55	60 65 70	75
· · · · · · · · · · · · · · · · · · ·			
			260
			240
			235
		· · · · · · · · · · · · · · · · · · ·	230
			220
		×	2/5
			210
			205
			200
E CROSS SECTIO	N ALONG BENT	LINE DRAWN	200
OGRAPHIC DATA FROM MOTT MA	FROM ELECTRO CDONALD DATE	DNIC FILES D MARCH 2017.	195
STRATIGRAPHY I. THE BORINGS WI	S DRAWN TH BOTH		
UNIU THE CRO EW: 90 DEGREES	SS SECTION.		190
40 45	50 55	60 65 70	75

GEOTECHNICAL BORING REPORT BORE LOG

١	VBS	17BP	.5.R.70			ТІ	P SF-340	0028	COUNT	Y FRANKL	IN			GEOLOGIST Hill, J. / G	Goodnight, D.		WBS	17BP.	.5.R.70			Т	P SF-3400)28	COUNT	Y	
SITE DESCRIPTION Br. No. 28 on SR 1210 (Montgomery Rd) over E								ontgomery	/ Rd) over	r Billys Creek GROUND WTR (ft)						SITE	DESCR	IPTION	Br. I	No. 28	3 on SI	n SR 1210 (Montgomery Rd) ove					
E	BORI	IG NO.	EB1-	A		S	TATION	13+70		OFFSET 9 ft LT				ALIGNMENT -L-	0 HR.	9.3	BOR	ING NO.	EB1-	·B		S	STATION 13+70				
(OLL	AR ELE	EV . 22	24.7 ft		т	OTAL DEP	TH 26.3	ft	NORTHING 873,579				EASTING 2,173,761	24 HR.	FIAD	COL	LAR ELE	EV . 22	24.8 ft		Т	OTAL DEPT	H 29.2 ft			
[RILL	RIG/HAI	MMER E	FF./DA	TE T	RI0055	CME-55 779	% 02/22/201	16		DRILL I	NETHC	DD H	H.S. Augers	HAMMER TYPE	Automatic	DRILL	RIG/HAI	MMER E	FF./DA	TE TI	RI0055	CME-55 77%	02/22/2016			
[RILL	ER T	OOTH	MAN, F	R.	S	TART DAT	E 04/20/	17	COMP. DA	TE 04/	20/17		SURFACE WATER DEP	TH N/A		DRIL	LER T	OOTHN	MAN, F	R.	S	FART DATE	04/26/1	7	C	
E	LEV	DRIVE ELEV	DEPTH	BLC	ow co	UNT		BLOWS	PER FOO	г	SAMP.	▼⁄		SOIL AND ROO	K DESCRIPTION		ELEV	DRIVE ELEV	DEPTH	BLC		UNT	BLOWS PER			ſ	
_	(ff)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0	25	50	75 100	NO.	Имо	I G	ELEV. (ft)		DEPTH (ft)	(π)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0 2	5 5	0	75	
	225												1 0 *	224.7		0.0	225				<u> </u>	ļ	ļ				
	-	223.7 -	- 1.0 -	2	2	2				· · · · · ·		м		RED-BROWN, CLA	EMBANKMENT YEY SILTY CSE. T	OF			- 1.0 -	1	1	1	\bullet_2 · · ·			:	
	220	221.2	3.5	woн	2	2	<mark> </mark> · · · ·			· · · · · · ·	<u>SS-1</u>	18%) (A-2-4) EMBANKMENT	<u> 3.0</u>	220	221.3	3.5	WOH	2	1			· · · · ·	:	
		218.7 -	6.0								00-1	1 10 70		219.2 BROWN, SANDY CL	AY (A-6) WITH TF AVEL	RACE		218.8 -	6.0								
		216.2	-	2	2	2	4			 		M	N N N					216.3	85		3	2	9 ⁵			:	
	215		- 0.0	3	2	4		·				w	000		Γ (A-5)		215		-	WOH	1	WOH	•1	• • • •	· · · ·	· 	
		-	+				!:::			· · · · · ·			000	TAN, F. TO CS	SE. SAND (A-1-b)			-	ŧ.							:	
	210	211.2	13.5	WOH	1	2	 		· · · · ·	· · · · · ·		0.04		GRAY, SILT	Y SAND (A-2-4)		210	211.3	13.5	2	2				· · · ·	:	
	210	-	ł		·		¶ <u>3</u>	+ • • • • • • • • • • • • • • • • • • •				Sat.		ین ۱۹۹۰ - ۲۰۰۰ ۱۹۹۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰ - ۲۰۰۰			210	-	F.	-	_		Q 3			. †	
		-	40.5							 			000	<u>207.7</u>	SE. SAND (A-1-b)	<u> </u>			105							:	
	205	206.2	18.5	1	3	4	↓ · · · ·	· · · ·				Sat.	000				205		10.5	2	3	5		· · ·	· · · ·	•	
		-	ŧ										000	0- 202.7		22.0		-	ŧ							:	
		201.2	23.5	100/0 /										WEATHE	RED ROCK GRANITE			201.3	23.5	25	39	45					
	200	100 4 -		100/0.4	1			+ • • • •		100/0.4				- 108 4		26.3	200		F	25	50	43		<u> </u>	<u></u>		
		- 190.4		60/0.0						60/0.0			90120	Boring Terminated	d WITH STANDAR	RD		-								:	
		-	L											Elevation 198.4 ft	t ON CR: GRANIT	E		196.3	- 28:5	100/0.3	<u>i</u>	<u> </u>	<u> </u>		<u> </u>	<u>. </u>	
		-	Ł											Ł				-	Ł	60/0.0							
		-	L											Ł				-	Ł								
		-	L											E				_	Ł								
		-	L											Ł				-	Ł								
		_	Ľ											-				-	E								
		-	Ē											E				-	E								
		-	F											F				-	F								
		-	F											F				_	F								
		-	F											F				-	F								
		-	F											F				-	F								
17		-	F											F				-	F								
1/30/		-	t -											F				-	ŧ								
0T 1		-	+											<u>L</u>				-	ŧ.								
DT.GI		-	t t											Ę				-	ŧ								
о С		-	t t											Ę				-	t t								
P. N		-	F											F				-	ŧ								
9.0 0		-	ŧ.											Ę				-	ŧ.								
CLIN C		-	Ļ											<u>-</u>				-	F.								
RANK		-	+											Ę				-	ł								
28 FF		-	L											È				-	ŧ								
BR		-	F											F					F								
UBL		-	Ł											Ł				-	Ł								
EDC		-	Ł											Ł				-	Ł								
BOR		-	Ł											Ł				-	Ł								
DOT		-	E											E				-	E								
ž		-																	Γ				L				

SHEET 7

INT	FRANKLI	N			GEOLOG	GIST	Hill, J. / (Goodnig	ght, D.	
ver	Billys Creek								GROUN	D WTR (ft)
	OFFSET 1	2 ft RT			ALIGNM	ENT	-L-		0 HR.	2.0
	NORTHING	873 5	91		FASTIN	G 21	73 778		24 HR	FIAD
				ם ח		• <u></u>	. 0,110	НАММ		Automatic
				u n.c						nuiomatic
	CONP. DA		20/17	L	SURFAC		IER DEP	TH N/	А	
501	75 100	SAIVIP.		0		SO	L AND RO	CK DESC	CRIPTION	
		UNU.		G						
					.224.8					0.0
			м		BF	ROWN,	SANDY C	LAY (A-6) WITH TR	ACE
· · · ·		66.2	10%				Gł	RAVEL		
	<u> </u>	00-2	1970		219.3					5.5
•••			М		216.8 T.		D GRAY, F	. SANDY	CLAYEY S	SILT8.0
			Sat.	000	 		5) WITH TI O GRAY, F.	TO CSE	. SAND (A	
										,
• •										
	· · · ·		Sat.							
•••										
•••				000	005.0					10.5
	+ • • • •		W		205.3					<u> </u>
				000	۱ ۲ <u>۵۵٬۰۰</u> ۰ ۱	VHITE	AND GRA	Y, SILTY	SAND (A-2	2-4) <u>- 21.0</u>
÷.,					200.3	TAN A	ND BROW	N, F. TO	CSE. SAN	ID 24 5
	1 83.— –		IVI	11			(/ WEATHI	4-1-b) E RED RO	ск	
							TAN,	GRANITI	Ξ	
	100/0.3				195.6					29.2
	60/0.0				•	Boring	g Terminate ETRATION	ed WITH TEST R	STANDAR EFUSAL at	D
						Eleva	tion 195.6	ft ON CR	: GRANITE	
				F						
				F						
				Ŀ						
				-						
				F						
					•					
				F						
				Ŀ						
				F						
				Ŀ						
				F						
				ļĘ						
				F						
				Ŀ						
				Γ						

GEOTECHNICAL BORING REPORT BORE LOG

v	/BS	17BP	.5.R.70)		TI	P SF-34	0028	COUNT	ry Franki	IN			GE	OLOGIST Hill, J. /	Goodnig	iht, D.	WB	S 17BF	P.5.R.70)		TI	P SF-3	40028		COUNT
SITE DESCRIPTION Br. No. 28 on SR 1210 (Montgomery Rd) over									Rd) over	Billys Creek							GROUND WTR (f) SIT	E DESCR	RIPTIO	N Br.I	on SF	SR 1210 (Montgomery Rd) ove				
В	ORI	NG NO.	. EB2-	-A		S	TATION	14+31		OFFSET 10 ft LT				AL	IGNMENT -L-		0 HR. 8.	BO	RING NO). EB2	-В	S	STATION 14+31				
С	OLL	AR ELI	EV . 22	24.7 ft		Т	OTAL DE	PTH 26.0	ft	NORTHING 873,627			EA	EASTING 2,173,724 24 HR. FIAD			o co	COLLAR ELEV. 224.7 ft TOTAL DEPTH 32.1 ft									
D	RILL	RIG/HA	MMER E	FF./DA	TE TF	RI0055	CME-55 77	7% 02/22/201	6		DRILL METHOD H.				ers	HAMM	ER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE TRI0055 CME-55 77% 02/22/2016									
D	RILL	ER T		MAN, I	٦.	S		FE 04/20/	17	COMP. DA	TE 04/	/20/17	7	SU	RFACE WATER DEF	PTH N/	A	DRI	LLER 1		MAN, F	२.	S		ATE 0	4/20/17	7
EL	.EV	DRIVE	DEPTH	BLC				BLOWS	PER FOO	T 75 100	SAMP.	· V /			SOIL AND RO	CK DESC	RIPTION	ELEV		DEPTH				0	BL	OWS P	ER FOOT
_	,	(ft)	(11)	0.5π	0.511	0.511		25	50	15 100	NO.	ИМС	DI G	ELEV	′. (ft)		DEPTH	ft) (IV)	(ft)	(11)	0.5π	0.51	0.5π		25		0
2	25	223.7 -	- 1.0				 1							224.7	ROADWAY	EMBAN		<u>.0</u> 225	223.7	- 1.0				1			
		-	- 25	2	2	1	♦ 3 : :	. .	· · · ·	 		M		<u>.</u>	BROWN-ORANGE WITH TR	E, SILTY ACE GRA	SAND (A-2-4) AVEL		001.0	+ ~ ~	2	1	1	•2 · ·	· · ·	 	· · · ·
2	20		- 3.5	WOH	3	3		· · · · ·		· · · · ·		м		- 219.2	!		5	.5 220		- 3.5	1	2	1		• •		
	-	218.7 -	+ 6.0 +	1	1	3		· · · · · ·		· · · · · ·	SS-3	24%	,				— — — — — — — — — —		218.7	+ 6.0 +	1	1	2		: :	· · ·	
2	15	216.2	8.5	2	2	2		· · · · · ·		· · · · · ·		Sat	000	<u>216.7</u>		SE. SAN	D (A-1-b)	215	216.2	8.5	3	3	2		· · ·	· · ·	
_		-	ŧ											2						Ŧ							
		211.2	13.5			-							000	ξ.					211.2	+ + 13.5			-				
2	10	-	Ŧ	2	4	6	•10_		· · ·			Sat.	. 000	E				210		Ŧ	2	2	2	4		· · · ·	
			Ī										000							Ī							
2	05	206.2	18.5	1	3	4	. ! ∳7					Sat.		Ł				205	206.2	18.5	1	2	2	i			
		-	ŧ					. .	· · ·				000	- 202 2			2	5		ŧ				<u> :</u> : :	: :	· · ·	
2	00	201.2	23.5	22	34	66.3	.j . <u></u> .	 <u>- </u>		· · · · · ·			000	202.2			24	0 200	201.2	23.5	25	61	39	<u>.</u>	÷+:	: <u> </u>	
		198.7 -	26.0			00.0				100/0.8				1 198.7	WEATH		<u>pck</u> <u>26</u>	.0		+							
		-	ŧ	60/0.0						00/0.0				F	Boring Terminate	ed WITH	STANDARD		196.2	+ 28.5						· · ·	
		-	ŧ											F	PENETRATION Elevation 198.7	I TEST R ft ON CR	EFUSAL at : GRANITE	195		+	100/0.5						
		-	Ŧ											E					192.7	32.0	60/0 1						
		-	I											E						Ī	00/011						
		-	ŧ											F						ŧ							
		-	ŧ											Ę						‡							
		-	ŧ											F					-	‡							
		-	ŧ											Ę						‡							
		-	ŧ											F						‡							
		-	ŧ											F						ŧ							
		-	Ŧ											F						Ŧ							
17		-	Ŧ											F					-	Ŧ							
11/30/		-	Ŧ											E						I							
DT 1		-	Ŧ											E					-	Ŧ							
00T.0		-	ŧ											Ł						Ŧ							
		-	ŧ											F						1							
GPJ		-	ŧ											F						ŧ							
N CO		-	ŧ											Ę						ŧ							
NKLI		-	ŧ											F					-	ŧ							
8 FRA		-	ŧ											Ę						‡							
BR2(-	‡											F					.	‡							
JBLE		-	ŧ											F						‡							
DO		-	ŧ											F						Ŧ							
BOR		-	Ŧ											E					-	Ŧ							
CDOT		-	Ŧ											E						Ŧ							
ž		-					l														1						

SHEET 8

